Organisation intergouvernementale pour les transports internationaux ferroviaires

Zwischenstaatliche Organisation für den internationalen Eisenbahnverkehr

Intergovernmental Organisation for International Carriage by Rail

Uniform Technical Prescription

Subsystem: INFRASTRUCTURE

UTP INF

Applicable from 01.01.2026

APTU Uniform Rules (Appendix F to COTIF 1999)

Uniform Technical Prescription applicable to the subsystem: "INFRASTRUCTURE"

(UTP INF)

This UTP has been developed in accordance with COTIF 1999 as last modified on 1 November 2023 and in particular with Articles 3, 4, 6, 7, 7a and 8 of the APTU Uniform Rules (Appendix F to COTIF).

For definitions, see also Article 2 of the APTU Uniform Rules and Article 2 of the ATMF Uniform Rules (Appendix G to COTIF).

Footnotes are not legal provisions. They include both explanatory information and references to other regulations.

0. EQUIVALENCE AND APPLICATION

(1) Following their adoption by the Committee of Technical Experts, the OTIF provisions included in this UTP are declared as being equivalent to the corresponding European Union (EU) regulations within the meaning of Article 13 § 4 letter b) of the APTU UR and Article 3a of the ATMF UR, in particular:

- Commission Regulation (EU) No 1299/2014 of 18 November 2014, as last amended by Commission Implementing Regulation (EU) 2023/1694 of 10 August 2023, hereinafter referred to as the INF TSI.

The equivalence is limited to the technical provisions (chapters 4 and 5) and the particular assessment procedures (point 6.2.4).

(2) In accordance with Article 8 § 2 of the ATMF UR, admission of infrastructure and supervision of its maintenance remain subject to the provisions in force in the Contracting State in which the infrastructure is located. The specifications in this UTP which are concerned by this Article of the

ATMF UR are therefore non-binding but recommended.

(3) Where provisions in this UTP and the INF TSI differ in substance, the respective texts are in a 2-column format. The left-hand column and the full width texts show the UTP provisions and the right-hand column shows the European Union TSI text. The list of specific cases in section 7.7 for Contracting States that are not members of the European Union are in full-width.

Texts in the right-hand column are strictly for information only. For EU law, consult the Official Journal of the European Union.

Where differences between texts of this UTP and the INF TSI are editorial, or not substantive, or concern the list of terms quoted below, the INF TSI texts are not generally reproduced. The TSI texts may however be reproduced in some instances to improve clarity and readability.

(4) The purpose of this UTP is to promote compatibility between neighbouring lines and networks, without compromising coherence between the international lines and the domestic network.

In order not to hinder states in ensuring such coherence, states may decide on a line-by-line basis whether or not to apply this UTP.

Infrastructure in the context of this UTP only covers parameters that are relevant in terms of technical compatibility with vehicles, so it does not therefore constitute an exhaustive design specification.

(5) States may declare that a particular line complies with this UTP. It shall do so only if the design, construction and conformity assessment of the line comply with all provisions of this UTP.

(6) The subsystem “infrastructure” is one of the three fixed installation subsystems defined in UTP GEN-B: the other subsystems are the energy subsystem and the trackside control-command and signalling subsystem. This document concerns the *subsystem infrastructure* only.

(7) The objectives and scope of COTIF and the EU law concerning railways are not identical and it has

therefore been necessary to use different terminology for concepts that have a similar but not identical meaning. The following table lists the terms used in this UTP and the corresponding terms used in the INF TSI:

This UTP	INF TSI
admission	authorisation
Declaration of conformity	EC declaration of conformity
Type examination	EC type examination
UTP verification procedure	EC verification
UTP declaration of verification	EC declaration of verification
UTP Certificate of verification	EC Certificate of verification
Contracting State	Member State
Assessing Entity	Notified Body

1. INTRODUCTION

1.1 Technical scope

This UTP concerns the infrastructure subsystem in accordance with UTP GEN-B.

This UTP sets out the railway infrastructure parameters that are relevant in terms of compatibility with vehicles and specific methods to check these parameters.

This TSI concerns the infrastructure subsystem and part of the maintenance subsystem of the Union rail system in accordance with Article 1 of Directive (EU) 2016/797[1].

The infrastructure and the maintenance subsystems are defined respectively in points 2.1 and 2.8 of Annex II to Directive (EU) 2016/797.

[1] Directive (EU) 2016/797 of the European Parliament and of the Council of 11 May 2016 on the interoperability of the rail system within the European Union

This UTP contains provisions concerning the following nominal track gauges: 1 435 mm, 1 520 mm, 1 524 mm, 1 600 mm and 1 668 mm.

Metric gauge is excluded from the technical scope.

The technical scope of this TSI is further defined in Article 2(1), 2(5) and 2(6) of this Regulation[2].

1.2 Geographical scope

The geographical scope of this UTP is defined by the lines open to, or used for international traffic in accordance with the ATMF UR.

Contracting States on whose territory a line is located shall, for their territory, decide whether this UTP is applicable to that line.

Contracting States are recommended to apply this UTP on all new lines which will be open for international traffic and on existing lines which are substantially used for international traffic, if such lines are upgraded or renewed, as described in 7.3.

Neighbouring Contracting States are recommended to coordinate the application of this UTP where relevant.

The geographical scope of this TSI is defined in Article 2(4) of this Regulation[3].

1.3 Content of this UTP

(1) The substance of this UTP is developed in accordance with Article 8 § 4 letters a) to i) and Article 8 § 6 of the APTU UR.

In accordance with Article 4(3) of Directive (EU) 2016/797, this TSI:

(a) indicates its intended scope (section 2);

(b) lays down essential requirements for the infrastructure and part of the maintenance subsystems (section 3);

(c) establishes the functional and technical specifications to be met by the infrastructure and part of the maintenance subsystems and its interfaces vis-à-vis other subsystems (section 4);

(d) specifies the interoperability constituents and interfaces which must be covered by European specifications, including

[2] Enacting part of the INF TSI

[3] Enacting part of the INF TSI:

"The TSI shall apply to the network of the Union rail system as described in Annex I of Directive (EU) 2016/797 with the exclusion of cases referred to in Article 1 (3) and (4) of Directive (EU) 2016/797."

European standards, which are necessary to achieve interoperability within the Union rail system (section 5);

(e) states, in each case under consideration, which procedures are to be used in order to assess the conformity or the suitability for use of the interoperability constituents, on the one hand, or the EC verification of the subsystems, on the other hand (section 6);

(f) indicates the strategy for implementing this TSI (section 7);

(g) indicates, for the staff concerned, the professional qualifications and health and safety conditions at work required for the operation and maintenance of the infrastructure subsystem, as well as for the implementation of this TSI (section 4).

(h) indicates the provisions applicable to the existing infrastructure subsystem, in particular in the event of upgrading and renewal and, in such cases, the modification work which requires an application for a new authorisation;

(i) indicates the parameters of infrastructure subsystem to be checked by the railway undertaking and the procedures to be applied to check those parameters after the delivery of the vehicle authorisation for placing on the market and before the first use of the vehicle to ensure compatibility between vehicles and the routes on which they are to be operated.

In accordance with Article 4(5) of the Directive (EU) 2016/797, provisions for specific cases are indicated in section 7.

(2) Requirements in this UTP are valid for all track gauge systems within the scope of this UTP, unless a paragraph refers to specific track gauge systems or to specific nominal track gauges.

2. DEFINITION AND SCOPE OF SUBSYSTEM

2.1 Definition of the infrastructure subsystem

This UTP covers:

a) the infrastructure structural subsystem;

b) facilities for servicing trains

b) the part of the maintenance functional subsystem relating to the infrastructure subsystem

(that is: washing plants for external cleaning of trains, water restocking, refuelling, fixed installations for toilet discharge and electrical shore supplies).

The elements of the infrastructure subsystem are described in

UTP GEN-B (2.1. Infrastructure).

Point 2.1 of Annex II to Directive (EU) 2016/797.

The elements of the maintenance subsystem are described in point 2.8 of Annex II to Directive (EU) 2016/797.

The scope of this UTP therefore includes the following aspects of the infrastructure subsystem:

a) Line layout;

b) Track parameters;

c) Switches and crossings;

d) Track resistance to applied loads;

e) Structures resistance to traffic loads;

f) Immediate action limits on track geometry defects;

g) Platforms;

h) Health, safety and environment;

i) Provision for operation;

j) Fixed installations for servicing trains.

But only to the extent that interfaces with vehicles are concerned.

Further details are set out in point 4.2.2.

2.2 Interfaces of this UTP with other UTPs

Point 4.3 sets out the functional and technical specification of the interfaces with the other subsystems.

Point 4.3 of this TSI sets out the functional and technical specification of the interfaces with the following subsystems, as defined in the relevant TSIs:

a) Rolling stock subsystem,

b) Energy subsystem,

c) Control command and signalling subsystem,

d) Traffic operation and management subsystem.

Interfaces with the UTP concerning accessibility for Persons with Reduced Mobility (UTP PRM) are described in point 2.3 below.

Interfaces with the Safety in Railway Tunnels are described in point 2.4 below.

2.3 Interfaces with the UTP concerning accessibility for persons with reduced mobility

The requirements related to accessibility for persons with disabilities and persons with reduced mobility are set out in the UTP PRM. Point 2.4.9 of this UTP INF and its sub-points lay down requirements for platforms. Platforms interface with the rolling stock subsystem concerning vehicle access and egress.	All requirements relating to the infrastructure subsystem for the access of persons with reduced mobility to the railway system are set out in the Persons with Reduced Mobility TSI.

2.4 Interfaces with safety in railway tunnels

Contracting States shall ensure that railway tunnels used for international transport are sufficiently safe for use by vehicles which are admitted to international traffic. States shall apply the Uniform Technical Prescriptions for this purpose, where they exist.	All requirements relating to the infrastructure subsystem for safety in railway tunnels are set out in the Safety in Railway Tunnels TSI.

2.5 Relation to the safety management system

Contracting States shall ensure that the entity that manages infrastructure to which this UTP is applied has implemented processes which ensure continued compliance with this UTP, including operational and technical interfaces. If so required by applicable law in the state concerned, compliance shall be ensured by means of a safety management system.	Necessary processes to manage safety and operations according to the requirements in the scope of this TSI, including interfaces to humans, organisations or other technical systems, shall be designed and implemented in the infrastructure manager's safety management system as required by Directive(EU) 2016/798[4].

2.6 Relation to the codification of combined transport

(1) The provisions for structure gauge are laid down in point 4.2.3.1.

(2) The codification system used for the conveyance of intermodal loading units in combined transport shall be in accordance with the specification referenced in Appendix T, index [A]. It can be based on:

(a) the characteristics of the line and the exact position of the obstacles;

(b) the reference profile of the structure gauge of that line;

(c) a combination of the methods referred to in points (a) and (b).

[4] Directive (EU) 2016/798 of the European Parliament and of the Council of 11 May 2016 on railway safety.

3. ESSENTIAL REQUIREMENTS

The following table indicates basic parameters of this UTP and their correspondence to the essential requirements as set out and numbered in

UTP GEN-A 2017. | Annex III to Directive (EU) 2016/797.

Table 1 Basic Parameters of the infrastructure subsystem corresponding to the essential requirements

UTP point	Title of UTP point	Safety	Reliability Availability	Health	Environmental protection	Technical compatibility	Accessibility
4.2.3.1	Structure gauge	1.1.1, 2.1.1				1.5	
4.2.3.2	Distance between track centres	1.1.1, 2.1.1				1.5	
4.2.3.3	Maximum gradients	1.1.1				1.5	
4.2.3.4	Minimum radius of horizontal curve	1.1.3				1.5	
4.2.3.5	Minimum radius of vertical curve	1.1.3				1.5	
4.2.4.1	Nominal track gauge					1.5	
4.2.4.2	Cant	1.1.1, 2.1.1				1.5	1.6.1
4.2.4.3	Cant deficiency	1.1.1				1.5	
4.2.4.4	Abrupt change of cant deficiency	2.1.1					
4.2.4.5	Equivalent conicity	1.1.1, 1.1.2				1.5	
4.2.4.6	Railhead profile for plain line	1.1.1, 1.1.2				1.5	
4.2.4.7	Rail inclination	1.1.1, 1.1.2				1.5	
4.2.5.1	Design geometry of switches and crossings	1.1.1, 1.1.2, 1.1.3				1.5	
4.2.5.2	Use of swing nose crossings	1.1.2, 1.1.3					
4.2.5.3	Maximum unguided length of fixed obtuse crossings	1.1.1, 1.1.2				1.5	
4.2.6.1	Track resistance to vertical loads	1.1.1, 1.1.2, 1.1.3				1.5	
4.2.6.2	Longitudinal track resistance	1.1.1, 1.1.2, 1.1.3				1.5	
4.2.6.3	Lateral track resistance	1.1.1, 1.1.2, 1.1.3				1.5	

4.2.7.1	Resistance of new bridges to traffic loads	1.1.1, 1.1.3				1.5	
4.2.7.2	Equivalent vertical loading for new earthworks and earth pressure effects imposed on new structures	1.1.1, 1.1.3				1.5	
4.2.7.3	Resistance of new structures over or adjacent to tracks	1.1.1, 1.1.3				1.5	
4.2.7.4	Resistance of existing bridges and earthworks to traffic loads	1.1.1, 1.1.3				1.5	
4.2.8.1	The immediate action limit for alignment	1.1.1, 1.1.2	1.2				
4.2.8.2	The immediate action limit for longitudinal level	1.1.1, 1.1.2	1.2				
4.2.8.3	The immediate action limit for track twist	1.1.1, 1.1.2	1.2				
4.2.8.4	The immediate action limit of track gauge as isolated defect	1.1.1, 1.1.2	1.2				
4.2.8.5	The immediate action limit for cant	1.1.1, 1.1.2	1.2				
4.2.8.6	The immediate action limit for switches and crossings	1.1.1, 1.1.2	1.2			1.5	
4.2.9.1	Usable length of platforms	1.1.1, 2.1.1				1.5	
4.2.9.2	Platform height	1.1.1, 2.1.1				1.5	1.6.1
4.2.9.3	Platform offset	1.1.1, 2.1.1				1.5	1.6.1
4.2.9.4	Track layout alongside platforms	1.1.1, 2.1.1				1.5	1.6.1
4.2.10.1	Maximum pressure variations in tunnels	1.1.1, 2.1.1				1.5	
4.2.10.2	Effect of cross winds	1.1.1, 2.1.1	1.2			1.5	

4.2.10.3	Aerodynamic effect on ballasted track	1.1.1	1.2			1.5	
4.2.11.1	Location markers	1.1.1	1.2				
4.2.11.2	Equivalent conicity in service	1.1.1, 1.1.2				1.5	
4.2.12.2	Toilet discharge	1.1.5	1.2	1.3.1		1.5	
4.2.12.3	Train external cleaning facilities		1.2			1.5	
4.2.12.4	Water restocking	1.1.5	1.2	1.3.1		1.5	
4.2.12.5	Refuelling	1.1.5	1.2	1.3.1		1.5	
4.2.12.6	Electric shore supply	1.1.5	1.2			1.5	
4.4	Operating rules		1.2				
4.5	Maintenance rules		1.2				
4.6	Professional qualifications	1.1.5	1.2				
4.7	Health and safety conditions	1.1.5	1.2	1.3	1.4.1		

4. DESCRIPTION OF THE INFRASTRUCTURE SUBSYSTEM

4.1 Introduction

(1) The consistency between the different subsystems as defined in UTP GEN-B

The Union rail system, to which Directive (EU) 2016/797 applies and of which the infrastructure and maintenance subsystems are parts, is an integrated system whose consistency

needs to be verified. This consistency must be checked in particular with regard to the specifications of the infrastructure subsystem, its interfaces

with the other subsystems, in particular those of vehicles.

in relation to the other subsystems of the Union rail system in which it is integrated, as well as the operating and maintenance rules.

(2) The limiting values set out in this UTP are not intended to be imposed as usual design values. However, the design values must be within the limits set out in this UTP.

(3) The functional and technical specifications of the infrastructure and part of the maintenance subsystems and their interfaces, as described in points 4.2 and 4.3, do not impose the use of specific technologies or technical solutions, except where this is strictly necessary for

international traffic.

the interoperability of the Union rail system.

(4) Innovative solutions for interoperability which do not fulfil the requirements specified in this UTP and/or which are not assessable as stated in this UTP require new specifications and/or new assessment

methods. In order to allow technological innovation, these specifications and assessment methods shall be developed by the process for innovative solutions described in

6.1.3 of this UTP.	Article 10[5].

(5) Where reference is made to EN standards, any variations called 'national deviations' in the EN do not apply, unless otherwise specified in this UTP.

(6) Where line speeds are stated in (km/h) as a category or performance parameter in this UTP, it shall be allowed to translate the speed to equivalent (mph)

in states where this is the commonly used unit of speed.	as in Appendix G, for Ireland and for the networks of the United Kingdom in respect of Northern Ireland.

4.2 Functional and technical specifications of the infrastructure subsystem

4.2.1 UTP categories of line

(1)	In order to deliver interoperability cost-effectively this UTP defines performance levels for line categories.	The elements of the Union's rail network are set out in point 1 of Annex I to Directive (EU) 2016/797. In order to deliver interoperability cost-effectively, each element of the Union's rail network shall be assigned a "TSI category of line".
(2)	Lines shall be categorised in accordance with performance levels defined in this UTP. It	The TSI category of line

shall be a combination of traffic codes. For lines where only one type of traffic is carried (for example, a freight only line), a single code may be used to describe the performances; where mixed traffic runs the category will be described by one or more codes for passenger and freight. The combined traffic codes describe the envelope within which the desired mix of traffic can be accommodated.

(3) These UTP categories of line shall be used for the classification of existing lines to define a target system so that the relevant performance parameters will be met.

(4) Lines shall be classified based on the type of traffic (traffic code) characterised by the following performance parameters:

- structure gauge,
- axle load,
- line speed,
- train length,
- usable length of platform.

The structure gauge and the axle load values of a line are parameters that interface with the trains running on the line. For each of the traffic codes in Tables 2 and 3, the values in the columns "structure gauge" and "axle load" are mandatory minimum values.

[5] Article 10 of the enacting part of the INF TSI.

	Notwithstanding TEN-T requirements,

The range of values indicated in the columns for "line speed", "usable length of platform" and "train length" shall be applied, as long as reasonably practicable.

(5) The performance parameters listed in Table 2 and Table 3 are not intended to be used for compatibility checks between rolling stock and infrastructure. Route compatibility checks are subject to

the UTP TCRC concerning train composition and route compatibility checks.	point 4.2.2.5 and Appendix D.1 of the Annex of the Commission Implementing Regulation (EU) 2019/773[6] ("TSI OPE").

(6) Information defining minimum capability requirements for existing structures in relationship to different train types is given in Appendix E. For the networks of the United Kingdom in respect of Northern Ireland, information defining the relation between maximum axle load and maximum speed in accordance with type of vehicle is given in Appendix F.

(7) The performance levels for types of traffic are set out in Table 2 and Table 3.

Table 2 Infrastructure performance parameters for passenger traffic [7]
(The route compatibility checks are subject to UTP TCRC)

Traffic code	Structure gauge	Axle load (t)	Line speed (km/h)	Usable length of platform [m]
P1	GC	17 (*) / 21,5 (**)	250-350	400
P2	GB	20 (*) / 22,5 (**)	200-250	200-400
P3	DE3	22,5 (***)	120-200	200-400
P4	GB	22,5 (***)	120-200	200-400
P5	GA	20 (***)	80-120	50-200
P6	G1	12 (***)	n.a.	n.a.
P1520	S	22,5 (***)	80-160	35-400
P1600	IRL1	22,5 (***)	80-160	75-240

(*) Minimum required values of axle load to be used for checks of bridges using a dynamic appraisal, based on design mass in working order for power heads and locomotives and operational mass under normal payload for vehicles capable of carrying a payload of passengers or luggage (mass definitions in accordance with the specification referenced in Appendix T Index [1].

(**) Minimum required values of axle load to be used for checks of infrastructure using a static loading, based on design mass under exceptional payload for vehicles capable of carrying a payload of passengers or luggage (mass definitions in accordance with the specification referenced in Appendix T Index [1] with regard to the specification referenced in Appendix T Index [2]). This axle load may be linked to limited speed.

(***) To be used for checks of infrastructure used for static loading, based on design mass in working order for power heads and locomotives and design mass under exceptional payload for other vehicles (mass definitions in accordance with the specification referenced in Appendix T Index [1] with regard to the specification referenced in Appendix T Index [2]). This axle load may be linked to limited speed.

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6 Commission Implementing Regulation (EU) 2019/773 of 16 May 2019 on the technical specification for interoperability relating to the operation and traffic management subsystem of the rail system within the European Union.

7 Title of this table in the TSI: *Infrastructure performance parameters for passenger traffic (The route compatibility checks are subject to point 4.2.2.5 and Appendix D.1 of TSI OPE).*

Table 3 Infrastructure performance parameters for freight traffic [8]
(The route compatibility checks are subject to UTP TCRC)

Traffic code	Structure Gauge	Axle load (t)	Line speed (km/h)	Train length (m)
F1	GC	22,5 (*)	100-120	740-1050
F2	GB	22,5 (*)	100-120	600-1050
F3	GA	20 (*)	60-100	500-1050
F4	G1	18 (*)	n.a.	n.a.
F1520	S	25 (*)	50-120	1050
F1600	IRL1	22,5 (*)	50-100	150-450

(*) To be used for static checks of infrastructure, based on design mass in working order for power heads and locomotives and design mass under normal payload for other vehicles (mass definitions in accordance with the specification referenced in Appendix T Index [1]). This axle load may be linked to limited speed.

Note: Tables 2 and 3 are not to be used for compatibility checks between rolling stock and infrastructure.

Additional performance levels[9]:

Passenger code GCC-P:

- Gauge AAR Plate H
- Axle load 25 t
- Line speed 220 km/h
- Train length 400 m

Freight traffic code GCC-F:

- Gauge AAR Plate H
- Axle load 32.4 t
- Line speed 120 km/h
- Train length 2000 m

(8) For structures, axle load by itself is not sufficient to determine the requirements for infrastructure. Requirements are specified as follows:

- for new structures in points 4.2.7.1 and 4.2.7.2,
- for existing structures in point 4.2.7.4,
- for track in point 4.2.6.

(9) Passenger hubs, freight hubs and connecting lines are included in the above traffic codes, as appropriate.

[8] Title of this table in the TSI: *Infrastructure performance parameters for freight traffic (The route compatibility checks are subject to point 4.2.2.5 and Appendix D.1 of TSI OPE)*

[9] To be confirmed by GCC

(10) The objective of UTPs is to facilitate international traffic but not to limit other types of traffic or to limit infrastructure capacity.

This UTP shall not prevent Contracting States from using infrastructure for the movement of vehicles which do not fall within the scope of COTIF or are not being used in international traffic. Therefore Contracting States may construct new and upgraded lines able to accommodate:

In accordance with Article 4(7) of Directive (EU) 2016/797 which provides that TSIs shall not prevent the Member States from deciding on the use of infrastructures for the movement of vehicles not covered by the TSIs, it is allowed to design new and upgraded lines able to accommodate:

- gauges larger,
- axle loads higher,
- speeds greater,
- usable length of platform greater,
- trains longer

than those specified in Table 2 and Table 3.

(11) (Reserved)

(12) It is permissible for specific locations on the line to be designed for any or all of the performance parameters line speed, usable length of platform and train length less than those set out in Table 2 and Table 3, where duly justified to meet geographical, urban or environmental constraints.

4.2.2 Basic parameters characterising the infrastructure subsystem

4.2.2.1 List of Basic Parameters

The Basic Parameters characterising the infrastructure subsystem, grouped according to the aspects listed in point 2.1, are:

A. Line layout:

a) Structure gauge (4.2.3.1);

b) Distance between track centres (4.2.3.2);

c) Maximum gradients (4.2.3.3);

d) Minimum radius of horizontal curve (4.2.3.4);

e) Minimum radius of vertical curve (4.2.3.5).

B. Track parameters:

a) Nominal track gauge (4.2.4.1);

b) Cant (4.2.4.2);

c) Cant deficiency (4.2.4.3);

d) Abrupt change of cant deficiency (4.2.4.4);

e) Equivalent conicity (4.2.4.5),

f) Railhead profile for plain line (4.2.4.6);

g) Rail inclination (4.2.4.7).

C. Switches and crossings

a) Design geometry of switches and crossings (4.2.5.1);

b) Use of swing nose crossings (4.2.5.2);

c) Maximum unguided length of fixed obtuse crossings (4.2.5.3).

D. Track resistance to applied loads

a) Track resistance to vertical loads (4.2.6.1);

b) Longitudinal track resistance (4.2.6.2);

c) Lateral track resistance (4.2.6.3).

E. Structures resistance to traffic loads

a) Resistance of new bridges to traffic loads (4.2.7.1);

b) Equivalent vertical loading for new earthworks and earth pressure effects imposed on new structures (4.2.7.2);

c) Resistance of new structures over or adjacent to tracks (4.2.7.3);

d) Resistance of existing bridges and earthworks to traffic loads (4.2.7.4).

F. Immediate action limits on track geometry defects

a) The immediate action limit for alignment (4.2.8.1);

b) The immediate action limit for longitudinal level (4.2.8.2);

c) The immediate action limit for track twist (4.2.8.3);

d) The immediate action limit of track gauge as isolated defect (4.2.8.4);

e) The immediate action limit for cant (4.2.8.5);

f) The immediate action limits for switches and crossings (4.2.8.6).

G. Platforms

a) Usable length of platforms (4.2.9.1);

b) Platform height (4.2.9.2);

c) Platform offset (4.2.9.3);

d) Track layout alongside platforms (4.2.9.4).

H. Health, safety and environment

a) Maximum pressure variation in tunnels (4.2.10.1);

b) Effect of crosswinds (4.2.10.2);

c) Aerodynamic effect on ballasted track (4.2.10.3).

I. Provision for operation

a) Location markers (4.2.11.1);

b) Equivalent conicity in service (4.2.11.2).

J. Fixed installations for servicing trains

a) General (4.2.12.1);

b) Toilet discharge (4.2.12.2);

c) Train external cleaning facilities (4.2.12.3);

d) Water restocking (4.2.12.4);

e) Refuelling (4.2.12.5);

f) Electric shore supply (4.2.12.6).

K. Maintenance rules

a) Maintenance file (4.5.1);

b) Maintenance plan (4.5.2).

4.2.2.2 Requirements for Basic Parameters

(1) These requirements are described in the following points, together with any particular conditions that may be allowed in each case for the basic parameters and interfaces concerned.

(2) The values of basic parameters specified are only valid up to a maximum line speed of 350 km/h.

(3) For Ireland and for the United Kingdom in respect of Northern Ireland network the values of basic parameters specified are only valid up to a maximum line speed of 165 km/h.

(4) In case of multi-rail track, requirements of this UTP are to be applied separately to each pair of rails designed to be operated as separate track.

(5) Requirements for lines representing specific cases are described under point 7.7.

(6) A short section of track with devices to allow transition between different nominal track gauges is allowed.

(7) Requirements are described for the subsystem under normal service conditions. Consequences, if any, of the execution of works, which may require temporary exceptions as far as the subsystem performance is concerned, are dealt with in point 4.4.

(8) The performance levels of trains can be enhanced by adopting specific systems, such as vehicle body tilting. Special conditions are allowed for running such trains, provided they do not entail restrictions for other trains not equipped with such systems.

4.2.3 Line layout

4.2.3.1 Structure gauge

(1) The upper part of the structure gauge shall be set on the basis of the gauges selected according to point 4.2.1, which are set out in the specification referenced in Appendix T Index [3].

(2) The lower part of the structure gauge shall be GI2 as set out in the specification referenced in Appendix T Index [3]. Where tracks are equipped with rail brakes, structure gauge GI1 as set out in the same specification shall apply for the lower part of the gauge.

(3) Calculations of the structure gauge shall be done using the kinematic method in accordance with the requirements of the specification referenced in Appendix T Index [3].

(4) Instead of points (1) to (3), for the 1 520 mm track gauge system, all traffic codes selected according to point 4.2.1 are applied with the uniform structure gauge "S" as defined in Appendix H.

(5) Instead of points (1) to (3), for the 1 600 mm track gauge system, all traffic codes selected according to point 4.2.1 are applied with the uniform structure gauge IRL1 as defined in Appendix O.

4.2.3.2 Distance between track centres

(1) The distance between track centres shall be set on the basis of the gauges selected according to point 4.2.1.

(2) The nominal horizontal distance between track centres for new lines shall be specified for the design and shall not be smaller than the values from the Table 4; it considers margins for aerodynamic effects.

Table 4 Minimum nominal horizontal distance between track centres

Maximum allowed speed (km/h)	Minimum nominal horizontal distance between track centres (m)
$160 < v \leq 200$	3,80
$200 < v \leq 250$	4,00
$250 < v \leq 300$	4,20
$v > 300$	4,50

(3) The distance between track centres shall at least satisfy the requirements for the limit installation distance between track centres, defined in accordance with the specification referenced in Appendix T Index [3].

(4) Instead of points (1) to (3), for the 1 520 mm track gauge system, the nominal horizontal distance between track centres shall be specified for the design and shall not be smaller than the values from the Table 5; it considers margins for aerodynamic effects.

Table 5 Minimum nominal horizontal distance between track centres for the 1 520 mm track gauge system

Maximum allowed speed (km/h)	Minimum nominal horizontal distance between track centres (m)
$v \leq 160$	4,10
$160 < v \leq 200$	4,30
$200 < v \leq 250$	4,50
$v > 250$	4,70

(5) Instead of point (2), for the 1 668 mm track gauge system, the nominal horizontal distance between track centres for new lines shall be specified for the design and shall not be smaller than the values from the Table 6, it considers margins for aerodynamic effects.

Table 6 Minimum nominal horizontal distance between track centres for the 1 668 mm track gauge system

Maximum allowed speed (km/h)	Minimum nominal horizontal distance between track centres (m)
$160 < v \leq 200$	3,92
$200 < v \leq 250$	4,00
$250 < v \leq 300$	4,30
$300 < v \leq 350$	4,50

(6) Instead of points (1) to (3), for the 1 600 mm track gauge system, the distance between track centres shall be set on the basis of the gauges selected according to point 4.2.1. The nominal horizontal distance between track centres shall be specified for the design and shall not be less than 3,57 m for gauge IRL1; it considers margins for aerodynamic effects.

4.2.3.3 Maximum gradients

(1) Gradients of tracks through passenger platforms of new lines shall not be more than 2,5 mm/m, where vehicles are intended to be regularly attached or detached.

(2) Gradients of new stabling tracks intended for parking rolling stock shall not be more than 2,5 mm/m unless specific provision is made to prevent the rolling stock from running away.

(3) Gradients as steep as 35 mm/m are allowed for main tracks on new P1 lines dedicated to passenger traffic at the design phase provided the following ‘envelope’ requirements are observed:

a) the slope of the moving average profile over 10 km is less than or equal to 25 mm/m;

b) the maximum length of continuous 35 mm/m gradient does not exceed 6 km.

4.2.3.4 Minimum radius of horizontal curve

The minimum design radius of horizontal curve shall be selected with regard to the local design speed of the curve.

(1) The minimum horizontal design curve radius for new lines shall not be less than 150 m.

(2) Reverse curves, except in marshalling yards where wagons are shunted individually, with small radii for new lines shall be designed to prevent buffer locking.

For straight intermediate track elements between the curves, the specification referenced in Appendix T, Index [4] shall apply, whose values are based on the reference vehicles defined in the same specification. To prevent buffer locking for existing vehicles that do not fulfil the assumptions of the reference vehicles, the infrastructure manager may specify longer lengths for the straight intermediate element.

For non-straight intermediate track elements, a detailed calculation shall be made in order to check the magnitude of the end throw differences.

(3) Instead of point (2), for the 1 520 mm track gauge system, reverse curves with radii in the range from 150 m up to 250 m shall be designed with a section of straight track of at least 15 m between the curves.

4.2.3.5 Minimum radius of vertical curve

(1) The radius of vertical curves (except for humps in marshalling yards) shall be at least 500 m on a crest or 900 m in a hollow.

(2) For humps in marshalling yards the radius of vertical curves shall be at least 250 m on a crest or 300 m in a hollow.

(3) Instead of point (1), for the 1 520 mm track gauge system the radius of vertical curves (except the marshalling yards) shall be at least 5 000 m both on a crest and in a hollow.

(4) Instead of point (2), for the 1 520 mm track gauge system and for humps in marshalling yards the radius of vertical curves shall be at least 350 m on a crest and 250 m in a hollow.

4.2.4 Track parameters

4.2.4.1 Nominal track gauge

(1) European standard nominal track gauge shall be 1 435 mm.

(2) Instead of point (1), for the 1 520 mm track gauge system the nominal track gauge shall be 1 520 mm.

(3) Instead of point (1), for the 1 668 mm track gauge system, the nominal track gauge shall be 1 668 mm.

(4) Instead of point (1), for the 1 600 mm track gauge system the nominal track gauge shall be 1 600 mm.

4.2.4.2 Cant

(1) The design cant for lines shall be limited as defined in Table 7.

Table 7 Design cant (mm)

	Freight and mixed traffic	Passenger traffic
Ballasted track	160	180
Non ballasted track	170	180

(2) The design cant on tracks adjacent to station platforms where trains are intended to stop in normal service shall not exceed 110 mm.

(3) New lines with mixed or freight traffic on curves with a radius less than 305 m and a cant transition steeper than 1 mm/m, the cant shall be restricted to the limit given by the following formula:

$$D \leq (R - 50)/1{,}5$$

where D is the cant in mm and R is the radius in m.

(4) Instead of points (1) to (3), for the 1 520 mm track gauge system the design cant shall not exceed 150 mm.

(5) Instead of point (1), for the 1 668 mm track gauge system, the design cant shall not exceed 185 mm.

(6) Instead of point (2), for the 1 668 mm track gauge system, the design cant on tracks adjacent to station platforms where trains are intended to stop in normal service shall not exceed 125 mm.

(7) Instead of point (3), for the 1 668 mm track gauge system, for new lines with mixed or freight traffic on curves with a radius less than 250 m, the cant shall be restricted to the limit given by the following formula:

$$D \leq 0{,}9 * (R - 50)$$

where D is the cant in mm and R is the radius in m.

(8) Instead of point (1), for the 1 600 mm track gauge system the design cant shall not exceed 185 mm.

4.2.4.3 Cant deficiency

(1) The maximum values for cant deficiency are set out in Table 8.

Table 8 Maximum cant deficiency (mm)

Design speed (km/h)	v ≤ 160	160 < v ≤ 300	v > 300
For operation of rolling stock conforming to the UTP for locomotives and passenger rolling stock (UTP LOC&PAS)	153		100
For operation of rolling stock conforming to the UTP for freight wagons (UTP WAG)	130	–	–

(2) It is permissible for trains specifically designed to travel with higher cant deficiency (for example multiple units with axle loads lower than set out in table 2; vehicles with special equipment for the negotiation of curves) to run with higher cant deficiency values, subject to a demonstration that this can be achieved safely.

(3) Instead of point (1), for all types of rolling stock of the 1 520 mm track gauge system the cant deficiency shall not exceed 115 mm. This is valid for speeds up to 200 km/h.

(4) Instead of point (1), for the 1 668 mm track gauge system, the maximum values for cant deficiency are set out in Table 9.

Table 9 Maximum cant deficiency for the 1 668 mm track gauge system (mm)

Design speed (km/h)	v ≤ 160	160 < v ≤ 300	v > 300
For operation of rolling stock conforming to the UTP for locomotives and passenger rolling stock (UTP LOC&PAS)	175		115
For operation of rolling stock conforming to the UTP for freight wagons (UTP WAG)	150	–	–

4.2.4.4 Abrupt change of cant deficiency

(1) The maximum values of abrupt change of cant deficiency shall be:

a) 130 mm for v ≤ 60 km/h;

b) 125 mm for 60 km/h < v ≤ 200 km/h;

c) 85 mm for 200 km/h < v ≤ 230 km/h;

d) 25 mm for v > 230 km/h.

(2) Where v ≤ 40 km/h and cant deficiency ≤ 75 mm both before and after an abrupt change of curvature, the value of abrupt change of cant deficiency may be raised to 150 mm.

(3) Instead of points (1) and (2), for the 1 520 mm track gauge system the maximum values of abrupt change of cant deficiency shall be:

a) 115 mm for v ≤ 200 km/h;

b) 85 mm for 200 km/h < v ≤ 230 km/h;

c) 25 mm for v > 230 km/h.

(4) Instead of point (1), for the 1 668 mm track gauge system, the maximum design values of abrupt change of cant deficiency shall be:

a) 150 mm for v ≤ 45 km/h;

b) 115 mm for 45 km/h < v ≤ 100 km/h;

c) (399-v)/2,6 [mm] for 100 km/h < v ≤ 220 km/h;

d) 70 mm for 220 km/h < v ≤ 230 km/h;

e) Abrupt change of cant deficiency is not allowed for speeds of more than 230 km/h.

4.2.4.5 Equivalent conicity

(1) The limiting values for equivalent conicity quoted in Table 10 shall be calculated for the amplitude (y) of the wheelset's lateral displacement:

- $y = 3$ mm, if $(TG - SR) \geq 7$ mm
- $y = \left(\frac{(TG - SR) - 1}{2}\right)$, if $5 \text{ mm} \leq (TG - SR) < 7$ mm
- $y = 2$ mm, if $(TG - SR) < 5$ mm

where TG is the track gauge and SR is the distance between the flange contact faces of the wheelset.

(2) No assessment of equivalent conicity is required for switches and crossings.

(3) Design track gauge, rail head profile and rail inclination for plain line shall be selected to ensure that the equivalent conicity limits set out in Table 10 are not exceeded.

Table 10 Equivalent conicity design limit values

	Wheel profile
Speed range (km/h)	S 1002, GV 1/40
$v \leq 60$	Assessment not required
$60 < v \leq 200$	0,25
$200 < v \leq 280$	0,20
$v > 280$	0,10

(4) The following wheelsets, as defined in the specification referenced in Appendix T, index [6], shall be modelled passing over the designed track conditions (simulated by calculation in accordance with the specification referenced in Appendix T, Index [5]):

a) S 1002 with SR1;

b) S 1002 with SR2;

c) GV 1/40 with SR1;

d) GV 1/40 with SR2.

For SR1 and SR2 the following values apply:

a) For the 1 435 mm track gauge system SR1 = 1 420 mm and SR2 = 1 426 mm;

b) For the 1 524 mm track gauge system SR1 = 1 505 mm and SR2 = 1 511 mm;

c) For the 1 600 mm track gauge system SR1 = 1 585 mm and SR2 = 1 591 mm;

d) For the 1 668 mm track gauge system SR1 = 1 653 mm and SR2 = 1 659 mm.

(5) Instead of points (1) to (4), for the 1 520 mm track gauge system, no assessment of equivalent conicity is required.

4.2.4.6 Railhead profile for plain line

(1) The railhead profile shall be selected from the range set out in one of the specifications referenced in Appendix T, Index [7] and Index [8], or shall be in accordance with point (2).

(2) The design of railhead profiles for plain line shall comprise:

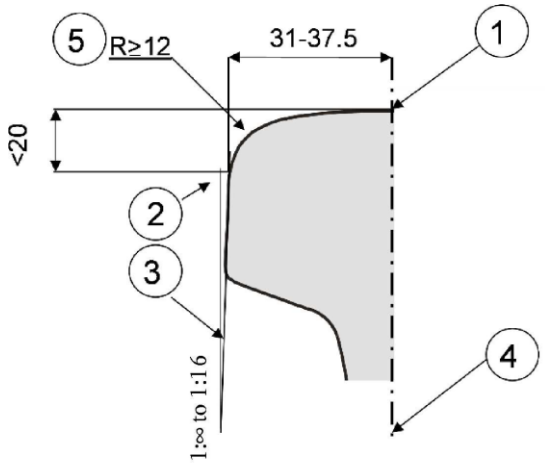
a) a lateral slope on the side of the railhead angled to between vertical and 1/16 with reference to the vertical axis of the railhead;

b) the vertical distance between the top of this lateral slope and the top of the rail shall be less than 20 mm;

c) a radius of at least 12 mm at the gauge corner;

d) the horizontal distance between the crown of the rail and the tangent point shall be between 31 and 37,5 mm.

Figure 1 Railhead profile



1 crown of rail
2 tangent point
3 lateral slope
4 vertical axis of rail head
5 gauge corner

(3) These requirements are not applicable to expansion devices.

4.2.4.7 Rail inclination

4.2.4.7.1 Plain line

(1) The rail shall be inclined towards the centre of the track.

(2) For tracks intended to be operated at speeds greater than 60 km/h, the rail inclination for a given route shall be selected from the range 1/20 to 1/40.

(3) For sections of not more than 100 m between switches and crossings without inclination where the running speed is no more than 200 km/h, the laying of rails without inclination is allowed.

4.2.4.7.2 Requirements for switches and crossings

(1) The rail shall be designed to be either vertical or inclined.

(2) If the rail is inclined, the designed inclination shall be selected from the range 1/20 to 1/40.

(3) The inclination can be given by the shape of the active part of the rail head profile.

(4) Within switches and crossings where the running speed is more than 200 km/h and no more than 250 km/h, the laying of rails without inclination is allowed provided that it is limited to sections not exceeding 50 m.

(5) For speeds of more than 250 km/h the rails shall be inclined.

4.2.5 Switches and crossings

4.2.5.1 Design geometry of switches and crossings

Point 4.2.8.6 defines immediate action limits for switches and crossings that are compatible with geometrical characteristics of wheelsets as defined in the rolling stock UTPs. It will be the task of the infrastructure manager to decide geometrical design values appropriate to its maintenance plan.

4.2.5.2 Use of swing nose crossing

For speeds higher than 250 km/h switches and crossings shall be equipped with swing-nose crossings.

4.2.5.3 Maximum unguided length of fixed obtuse crossings

The design value of the maximum unguided length of fixed obtuse crossings shall be in accordance with the requirements set out in Appendix J.

4.2.6 Track resistance to applied loads

4.2.6.1 Track resistance to vertical loads

The track design, including switches and crossings, shall take into account at least the following forces:

a) the axle load selected according to point 4.2.1;

b) maximum vertical wheel forces. Maximum wheel forces for defined test conditions are set out in the specification referenced in Appendix T, Index [9];

c) vertical quasi-static wheel forces. Maximum quasi-static wheel forces for defined test conditions are set out in the specification referenced in Appendix T, Index [9].

4.2.6.2 Longitudinal track resistance

4.2.6.2.1 Design forces

The track, including switches and crossings, shall be designed to withstand longitudinal forces equivalent to the force arising from braking of 2,5 m/s^2 for the performance parameters chosen in accordance with point 4.2.1.

4.2.6.2.2 Compatibility with braking systems

(1) The track, including switches and crossings, shall be designed to be compatible with the use of magnetic braking systems for emergency braking.

(2) Provisions for the use of eddy current braking systems on track shall be defined at operational level by the infrastructure manager on the basis of the specific characteristics of the track, including switches and crossings. The conditions of use of this braking system

shall be made available by the infrastructure manager to any railway undertaking operating on the infrastructure in accordance with the UTP	are registered in accordance with Commission Implementing Regulation (EU) 2019/777[10] (RINF).

[10] Commission Implementing Regulation (EU) 2019/777 of 16 May 2019 on the common specifications for the register of railway infrastructure.

TCRC concerning train composition and route compatibility checks.

(3) For the 1 600 mm track gauge system it shall be allowed not to apply point (1).

4.2.6.3 Lateral track resistance

The track design, including switches and crossings, shall take into account at least the following forces:

a) lateral forces; maximum lateral forces exerted by a wheel set on the track for defined test conditions are set out in the specification referenced in Appendix T, Index [9];

b) quasi-static guiding forces; maximum quasi-static guiding forces Y_{qst} for defined radii and test conditions are set out in the specification referenced in Appendix T, Index [9].

4.2.7 Structures resistance to traffic loads

The requirements of the specifications referenced in Appendix T, Index [10] and Index [11] specified in this point of the UTP are to be applied in accordance with the corresponding points in the national annexes to those specifications if they exist.

4.2.7.1 Resistance of new bridges to traffic loads

4.2.7.1.1 Vertical loads

(1) Bridges shall be designed to support vertical loads in accordance with the following load models, set out in the specification referenced in Appendix T, Index [10]:

a) Load Model 71, as set out in the specification referenced in Appendix T, Index [10];

b) In addition, for continuous bridges, Load Model SW/0, as set out in the specification referenced in Appendix T, Index [10].

(2) The load models shall be multiplied by the factor alpha (α) as set out in the specification referenced in Appendix T, Index [10].

(3) The value of factor alpha (α) shall be equal to or greater than the values set out in Table 11.

Table 11 Factor alpha (α) for the design of new bridges

Type of traffic	Minimum factor alpha (α)
P1, P2, P3, P4	1,0
P5	0,91
P6	0,83
P1520	1
P1600	1,1
F1, F2, F3	1,0
F4	0,91
F1520	1,46
F1600	1,1

4.2.7.1.2 Allowance for dynamic effects of vertical loads

(1) The load effects from the Load Model 71 and Load Model SW/0 shall be enhanced by the dynamic factor phi (Φ) as set out in the specification referenced in Appendix T, Index [10].

(2) For bridges for speeds over 200 km/h where the specification referenced in Appendix T, Index [10] requires a dynamic analysis to be carried out, the bridge shall additionally be designed for HSLM defined in the specification referenced in Appendix T, Index [10].

(3) It is permissible to design new bridges such that they will also accommodate an individual passenger train with higher axle loads than covered by HSLM. The dynamic analysis shall be undertaken using the characteristic value of the loading from the individual train taken as the design mass under normal payload in accordance with Appendix K with an allowance for passengers in standing areas in accordance with Note (1) of Appendix K.

4.2.7.1.3 Centrifugal forces

Where the track on a bridge is curved over the whole or part of the length of the bridge, the centrifugal force shall be taken into account in the design of bridges as set out in the specification referenced in Appendix T, Index [10].

4.2.7.1.4 Nosing forces

The nosing force shall be taken into account in the design of bridges as set out in the specification referenced in Appendix T, Index [10].

4.2.7.1.5 Actions due to traction and braking (longitudinal loads)

Traction and braking forces shall be taken into account in the design of bridges as set out in the specification referenced in Appendix T, Index [10].

4.2.7.1.6 Design track twist due to rail traffic actions

The maximum total design track twist due to rail traffic actions shall not exceed the values set out in the specification referenced in Appendix T, Index [11].

4.2.7.2 Equivalent vertical loading for new geotechnical structures, earthworks and earth pressure effects

(1) Geotechnical structures and earthworks shall be designed and earth pressure effects shall be specified taking into account the vertical loads produced by the Load Model 71, as set out in the specification referenced in Appendix T, Index [10].

(2) The equivalent vertical loading shall be multiplied by the factor alpha (α) as set out in the specification referenced in Appendix T, Index [10]. The value of α shall be equal to or greater than the values set out in Table 11.

4.2.7.3 Resistance of new structures over or adjacent to tracks

Aerodynamic actions from passing trains shall be taken into account as set out in the specification referenced in Appendix T, Index [10].

4.2.7.4 Resistance of existing structures (bridges, geotechnical structures and earthworks) to traffic loads

(1) Bridges, geotechnical structures and earthworks shall be brought to a specified level of interoperability in accordance with the UTP category of the line referred to in point 4.2.1.

(2) The minimum capability requirements for structures for each traffic code are given in Appendix E and must be met for the line to be declared interoperable.

(3) The following conditions apply:

a) Where an existing structure is replaced by a new structure then the new structure shall be in accordance with the requirements of point 4.2.7.1 or point 4.2.7.2.

b) If the minimum capability of the existing structures satisfies the requirements in Appendix E then the existing structures satisfy the relevant interoperability requirements.

c) Where the capability of an existing structure does not satisfy the requirements in Appendix E and works (e.g. strengthening) are being carried out to raise the capability of the structure to meet the requirements of this UTP (and the structure is not to be replaced by a new structure) then the structure shall be brought into conformity with the requirements in Appendix E.

(4) For the networks of the United Kingdom

in Great Britain and Northern Ireland | (Northern Ireland)

, in points (2) and (3) the EN line category may be replaced by Route Availability (RA) number (in accordance with the national technical rule notified for that purpose) and consequently references to Appendix E are replaced by references to Appendix F.

4.2.8 Immediate action limits on track geometry defects

4.2.8.1 The immediate action limit for alignment

(1) The immediate action limits for isolated defects in alignment are set out in the specification referenced in Appendix T, Index [12]. Isolated defects shall not exceed the limits of wavelength range D1.

(2) The immediate action limits for isolated defects in alignment for speeds of more than 300 km/h are an open point.

4.2.8.2 The immediate action limit for longitudinal level

(1) The immediate action limits for isolated defects in longitudinal level are set out in the specification referenced in Appendix T, Index [12]. Isolated defects shall not exceed the limits of wavelength range D1.

(2) The immediate action limits for isolated defects in longitudinal level for speeds of more than 300 km/h are an open point.

4.2.8.3 The immediate action limit for track twist

(1) The immediate action limit for track twist as an isolated defect is given as a zero to peak value. Track twist set out in the specification referenced in Appendix T, Index [13].

(2) The track twist limit is a function of the measurement base applied in accordance with the specification referenced in Appendix T, Index [12].

(3) The infrastructure manager shall set out in the maintenance plan the base-length on which it will measure the track in order to check compliance with this requirement. The base-length of measurement shall include at least one base between 2 and 5 m.

(4) Instead of points (1) and (2), for the 1 520 mm track gauge system the track twist, for a base length of 10 m, shall be not more than:

a) 16 mm for passenger lines with v > 120 km/h or freight lines with v > 80 km/h;

b) 20 mm for passenger lines with v ≤ 120 km/h or freight lines with v ≤ 80 km/h.

(5) Instead of point (3), for the 1 520 mm track gauge system the Infrastructure Manager shall set out in the maintenance plan the base-length on which it will measure the track in order to check compliance with this requirement. The base-length of measurement shall include at least one base of 10 m.

(6) Instead of point (2), for the 1 668 mm track gauge system, the track twist limit is a function of the measurement base applied in accordance with the specification referenced in Appendix T, Index [12].

4.2.8.4 The immediate action limit of track gauge as an isolated defect

(1) The immediate action limits of track gauge as an isolated defect are set out in Table 12.

Table 12 Immediate action limits of track gauge

Speed (km/h)	Dimensions (mm)	
	Minimum track gauge	Maximum track gauge
v ≤ 120	1 426	1 470
120 < v ≤ 160	1 427	1 470
160 < v ≤ 230	1 428	1 463
v > 230	1 430	1 463

(2) Instead of point (1), for the 1 520 track gauge system the immediate action limits of track gauge as an isolated defect are set out in Table 13.

Table 13 Immediate action limits of track gauge for 1 520 mm track gauge system

Speed (km/h)	Dimensions (mm)	
	Minimum track gauge	Maximum track gauge
v ≤ 140	1 512	1 548
v > 140	1 512	1 536

(3) Instead of point (1), for the 1 600 track gauge system the immediate action limits of track gauge as an isolated defect are:

a) minimum track gauge: 1 591 mm;

b) maximum track gauge: 1 635 mm.

4.2.8.5 The immediate action limit for cant

(1) The maximum cant allowed in service is 180 mm.

(2) The maximum cant allowed in service is 190 mm for dedicated passenger traffic lines.

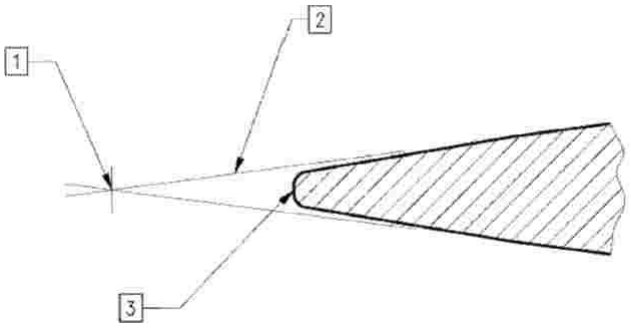
(3) Instead of points (1) and (2), for the 1 520 mm track gauge system, the maximum cant allowed in service is 150 mm.

(4) Instead of points (1) and (2), for the 1 600 mm track gauge system, the maximum cant allowed in service is 185 mm.

(5) Instead of points (1) and (2), for the 1 668 mm track gauge system, the maximum cant allowed in service is 200 mm.

4.2.8.6 The immediate action limits for switches and crossings

Figure 2. Point retraction in fixed common crossings



1 Intersection point (IP)

2 Theoretical reference line

3 Actual point (RP)

(1) The technical characteristics of switches and crossings shall comply with the following in-service values:

a) Maximum value of free wheel passage in switches: 1 380 mm.

This value can be increased if the infrastructure manager demonstrates that the actuation and locking system of the switch is able to resist the lateral impact forces of a wheelset.

b) Minimum value of fixed nose protection for common crossings: 1 392 mm.

This value is measured 14 mm below the running surface, and on the theoretical reference line, at an appropriate distance back from the actual point (RP) of the nose as indicated in Figure 2.

For crossings with point retraction, this value can be reduced. In this case the infrastructure manager shall demonstrate that the point retraction is sufficient to guarantee that the wheel will not hit the nose at the actual point (RP).

c) Maximum value of free wheel passage at crossing nose: 1 356 mm.

d) Maximum value of free wheel passage at check rail/wing rail entry: 1 380 mm.

e) Minimum flangeway width: 38 mm.

f) Minimum flangeway depth: 40 mm.

g) Maximum height of check rail: 70 mm.

(2) All relevant requirements for switches and crossings are also applicable to other technical solutions using switch rails, for example side modifiers used in multi-rail track.

(3) Instead of point (1), for the 1 520 mm track gauge system the technical characteristics of switches and crossings shall comply with the following in-service values:

a) Minimum value of bypass at the narrowest location between open switch rail and stock rail is 65 mm.

b) Minimum value of fixed nose protection for common crossings is 1 472 mm.

c) This value is measured 13 mm below the running surface, and on the theoretical reference line, at an appropriate distance back from the actual point (RP) of the nose as indicated in Figure 2. For crossings with point retraction, this value can be reduced. In this case the Infrastructure Manager shall demonstrate that the point retraction is sufficient to guarantee that the wheel will not hit the nose at the actual point (RP).

d) Maximum value of free wheel passage at crossing nose is 1 435 mm.

e) Minimum flangeway width is 42 mm.

f) Minimum flangeway depth is 40 mm.

g) Maximum height of check rail is 50 mm.

(4) Instead of point (1), for the 1 600 mm track gauge system the technical characteristics of switches and crossings shall comply with the following in-service values:

a) Maximum value of free wheel passage in switches: 1 546 mm. This value can be increased if the infrastructure manager demonstrates that the actuation and locking system of the switch is able to resist the lateral impact forces of a wheelset.

b) Minimum value of fixed nose protection for common crossings: 1 556 mm. This value is measured 14 mm below the running surface, and on the theoretical reference line, at an appropriate distance back from the actual point (RP) of the nose as indicated in Figure 2. For crossings with point retraction, this value can be reduced. In this case the infrastructure manager shall demonstrate that the point retraction is sufficient to guarantee that the wheel will not hit the nose at the actual point (RP).

c) Maximum value of free wheel passage at crossing nose: 1 520 mm.

d) Maximum value of free wheel passage at check rail/wing rail entry: 1 546 mm.

e) Minimum flangeway width: 38 mm.

f) Minimum flangeway depth: 40 mm.

g) Maximum height of check rail above head of running rail: 25 mm.

4.2.9 Platforms

(1) The requirements of this point are only applicable to passenger platforms where trains are intended to stop in normal service.

(2) For the requirements of this point it is permissible to design platforms required for the current service requirement provided provision is made for the reasonably foreseeable future service requirements. When specifying the interfaces with trains intended to stop at the platform, consideration shall be given

to both the current service requirements and the reasonably foreseeable service requirements at least 10 years following the bringing into service of the platform.

4.2.9.1 Usable length of platforms

The usable length of a platform shall be defined according to point 4.2.1.

4.2.9.2 Platform height

(1) The nominal platform height

recommended is	shall be

550 mm or 760 mm above the running surface for radii of 300 m or more.

(2) For smaller radii the nominal platform height may be adjusted depending on the platform offset to minimise the stepping distance between the train and the platform.

(3) (reserved)	For platforms where only passenger trains that are explicitly listed as excluded from the scope of Commission Regulation (EU) No 1302/2014 (LOC&PAS TSI)[11] in its point 1.1 are intended to stop in normal service, different provisions for the nominal platform height might apply.

(4) Instead of points (1) and (2), for the 1520 mm track gauge system the nominal platform height

is recommended to	shall

be 200 mm or 550 mm above the running surface. These values shall be considered with a tolerance of -10/+20 mm.

(5) Instead of points (1) and (2), for the 1 600 mm track gauge system the nominal platform height

is recommended to	shall

be 915 mm above the running surface.

4.2.9.3 Platform offset

(1) The distance between the track centre and the platform edge parallel to the running plane (b_q), as defined in the specification referenced in Appendix T, Index [3], shall be set on the basis of the installation limit gauge (b_{qlim}). The installation limit gauge shall be calculated on the basis of the gauge G1.

(2) The platform shall be built close to the gauge within a maximum tolerance of 50 mm. The value for b_q shall therefore respond to:

$$b_{qlim} \leq b_q \leq b_{qlim} + 50 \text{ mm}.$$

[11] Commission Regulation (EU) No 1302/2014 of 18 November 2014 as last amended by Commission implementing regulation (EU) 2023/1694 of 10 August 2023 concerning a technical specification for interoperability relating to the rolling stock — locomotives and passenger rolling stock subsystem of the rail system in the European Union.

(3) Instead of points (1) and (2), for the 1 520 mm track gauge system the platform offset shall be:

a) 1 920 mm for platforms with heights of 550 mm; and

b) 1 745 mm for platforms with height of 200 mm.

These values shall be considered with a tolerance of -10/+10 mm.

(4) Instead of points (1) and (2), for the 1 600 mm track gauge system the platform offset shall be 1 560 mm.

4.2.9.4 Track layout alongside platforms

(1) Track adjacent to the platforms for new lines shall preferably be straight, but shall nowhere have a radius of less than 300 m.

(2) No values are specified for an existing track alongside new, renewed or upgraded platforms.

4.2.10 Health, safety and environment

4.2.10.1 Maximum pressure variations in tunnels

(1) Any new tunnel or underground structure falling in the categories described in the specification referenced in Appendix T, Index [14], has to provide that maximum pressure variation, caused by the passage of a train running at the maximum allowed speed in the tunnel, do not exceed 10 kPa during the time taken for the train to pass through the tunnel.

(2) The requirement of point (1) has to be fulfilled along the outside of any train complying with the UTP LOC&PAS.

(3) In the case of upgrading or renewal of the infrastructure subsystem, an existing tunnel or underground structure intended to be operated at speeds greater than or equal to 200 km/h has to provide that the maximum pressure variation, caused by the passage of a train running at the maximum allowed speed in the tunnel, does not exceed 10 kPa during the time taken for the train to pass through the tunnel. The assessment has to be performed in accordance with the specification referenced in Appendix T, Index [14], or in point 6.2.4.12(1) when it is not possible to apply a simplified conformity assessment.

4.2.10.2 Effect of crosswinds

(1) A line is interoperable from the cross wind point of view if safety is ensured for a reference train running along that line under the most critical operational conditions.

(2) The rules for proving conformity shall take into account the characteristic wind curves of the reference trains defined in the UTP LOC&PAS.

(3) If safety cannot be achieved without mitigating measures, either due to the geographic situation or to other specific features of the line, the infrastructure manager shall take the necessary measures to maintain the safety, for example by:

- locally reducing train speeds, possibly temporarily during periods at risk of storms,
- installing equipment to protect the track section concerned from cross winds,
- other appropriate means.

(4) It shall be demonstrated that safety is achieved after measures taken.

4.2.10.3 Aerodynamic effect on ballasted track

(1) The aerodynamic interaction between rolling stock and infrastructure may cause the lifting and further blowing away of ballast stones from the track bed in plain line and switches and crossings (Ballast pick up). This risk shall be mitigated.

(2) The requirements for the infrastructure subsystem aimed at mitigating the risk for 'ballast pick up' apply only to lines intended to be operated at speed greater than 250 km/h.

(3) The requirements of point (2) above are an open point.

4.2.11 Provision for operation

4.2.11.1 Location markers

Location markers shall be provided at nominal intervals along the track of not more than 1000 m.

4.2.11.2 Equivalent conicity in service

(1) If ride instability is reported, the railway undertaking and the infrastructure manager shall localise the section of the line

according to the procedures applicable in the State concerned so as to execute an investigation | in a joint investigation

according point (2) and (3) hereafter.

Note: This joint investigation is also specified in point 4.2.3.4.3.2 of UTP LOC&PAS for action on rolling stock.

(2) The infrastructure manager shall measure the track gauge and the railhead profiles at the site in question at a distance of approximate 10 m. The mean equivalent conicity over 100 m shall be calculated by modelling with the wheelsets (a) – (d) mentioned in point 4.2.4.5(4) in order to check for compliance, for the purpose of the joint investigation, with the limit equivalent conicity for the track specified in Table 14.

Table 14 Equivalent conicity in service limit values for the track (for the purpose of joint investigation)

Speed range (km/h)	Maximum value of mean equivalent conicity over 100 m
$v \leq 60$	assessment not required
$60 < v \leq 120$	0,40
$120 < v \leq 160$	0,35
$160 < v \leq 230$	0,30
$v > 230$	0,25

(3) If the mean equivalent conicity over 100 m complies with the limit values in Table 14, a joint investigation by the railway undertaking and the infrastructure manager shall be undertaken to specify the reason for the instability.

4.2.12 Fixed installations for servicing trains

4.2.12.1 General

This point 4.2.12 sets out the infrastructure elements of the maintenance subsystem required for servicing trains.

4.2.12.2 Toilet discharge

Fixed installations for toilet discharge shall be compatible with the characteristics of the retention toilet system specified in the UTP LOC & PAS.

4.2.12.3 Train external cleaning facilities

(1) Where a washing plant is provided it shall be able to clean the outer sides of single or double-deck trains between a height of:

a) 500 to 3 500 mm for a single-deck train;

b) 500 to 4 300 mm for double-deck trains.

(2) The washing plant shall be designed so that trains can be driven through it at any speed between 2 km/h and 5 km/h.

4.2.12.4 Water restocking

(1) Fixed equipment for water restocking shall be compatible with the characteristics of the water system specified in the UTP LOC & PAS.

(2) Fixed equipment for the supply of water intended for human consumption shall be supplied with drinking water

in accordance with the provisions defining drinking water quality that are applicable in the state concerned.	meeting the requirements of Directive (EU) 2020/2184 of the European Parliament and of the Council[12].

(3) The materials used for the supply of water intended for human consumption to the rolling stock (e.g. tank, pump, piping, water tap and sealing material and quality) shall comply with the requirements that relate to water intended for human consumption.

4.2.12.5 Refuelling

Refuelling equipment shall be compatible with the characteristics of the fuel system specified in the UTP LOC & PAS.

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[12] Directive (EU) 2020/2184 of the European Parliament and of the Council of 16 December 2020 on the quality of water intended for human consumption.

4.2.12.6 Electrical shore supply

Where provided, electrical shore supply shall be by means of one or more of the power supply systems specified in the UTP LOC & PAS.

4.3 Functional and technical specifications of the interfaces

From the standpoint of technical compatibility, the interfaces of the infrastructure subsystem with the other subsystems are like described in the following points.

4.3.1 Interfaces with the rolling stock subsystem

Table 15 Interfaces with the subsystem "Rolling stock - locomotives and passenger rolling stock"

Interface	Reference in UTP INF	Reference in UTP LOC&PAS
Track gauge	4.2.4.1 Nominal track gauge 4.2.5.1 Design geometry of switches and crossings 4.2.8.6 The immediate action limits for switches and crossings	4.2.3.5.2.1 Mechanical and geometrical characteristics of wheelset 4.2.3.5.3 Automatic variable gauge systems
Gauge	4.2.3.1 Structure gauge 4.2.3.2 Distance between track centres 4.2.3.5 Minimum radius of vertical curve 4.2.9.3 Platform offset	4.2.3.1. Gauging
Axle load and axle spacing	4.2.6.1 Track resistance to vertical loads 4.2.6.3 Lateral track resistance 4.2.7.1 Resistance of new bridges to traffic loads 4.2.7.2 Equivalent vertical loading for new earthworks and earth pressure effects imposed on new structures 4.2.7.4 Resistance of existing bridges and earthworks to traffic loads	4.2.2.10 Load conditions and weighed mass 4.2.3.2.1 Axle load parameter
Running characteristics	4.2.6.1 Track resistance to vertical loads 4.2.6.3 Lateral track resistance 4.2.7.1.4 Nosing forces	4.2.3.4.2.1 Limit values for running safely 4.2.3.4.2.2 Track loading limit values
Ride stability	4.2.4.5 Equivalent conicity 4.2.4.6 Railhead profile for plain line 4.2.11.2 Equivalent conicity in service	4.2.3.4.3 Equivalent conicity 4.2.3.5.2.2 Mechanical and geometrical characteristics of wheels
Longitudinal actions	4.2.6.2 Longitudinal track resistance 4.2.7.1.5 Actions due to traction and braking (longitudinal loads)	4.2.4.5 Braking performance
Minimum horizontal curve radius	4.2.3.4 Minimum radius of horizontal curve	4.2.3.6 Minimum curve radius Annex A, A.1 Buffers
Running dynamic behaviour	4.2.4.3 Cant deficiency	4.2.3.4.2. Running dynamic behaviour
Maximum deceleration	4.2.6.2 Longitudinal track resistance 4.2.7.1.5 Actions due to traction and braking	4.2.4.5 Braking performance

Interface	Reference in UTP INF	Reference in UTP LOC&PAS
Aerodynamic effect	4.2.3.2 Distance between track centres 4.2.7.3 Resistance of new structures over or adjacent to tracks 4.2.10.1 Maximum pressure variations in tunnels 4.2.10.3 Aerodynamic effect on ballasted track	4.2.6.2.1 Slipstream effects on passengers on platforms and on trackside workers 4.2.6.2.2 Head pressure pulse 4.2.6.2.3 Maximum pressure variations in tunnels 4.2.6.2.5 Aerodynamic effect on ballasted tracks
Crosswind	4.2.10.2 Effect of crosswinds	4.2.6.2.4 Crosswind
Installations for servicing trains	4.2.12.2 Toilet discharge 4.2.12.3 Train external cleaning facilities 4.2.12.4 Water restocking 4.2.12.5 Refuelling 4.2.12.6 Electric shore supply	4.2.11.3 Toilet discharge system 4.2.11.2.2 Exterior cleaning through a washing plant 4.2.11.5 Interface for water refilling 4.2.11.7 Refuelling equipment 4.2.11.6 Special requirements for stabling of trains

Table 16 Interfaces with the subsystem "Rolling stock - freight wagons"

Interface	Reference in UTP INF	Reference in UTP WAG
Track gauge	4.2.4.1 Nominal track gauge 4.2.4.6 Railhead profile for plain line 4.2.5.1 Design geometry of switches and crossings 4.2.8.6 The immediate action limits for switches and crossings	4.2.3.6.2 Characteristics of wheelsets 4.2.3.6.3 Characteristics of wheels
Gauge	4.2.3.1 Structure gauge 4.2.3.2 Distance between track centres 4.2.3.5 Minimum radius of vertical curve 4.2.9.3 Platform offset	4.2.3.1. Gauging
Axle load and axle spacing	4.2.6.1 Track resistance to vertical loads 4.2.6.3 Lateral track resistance 4.2.7.1 Resistance of new bridges to traffic loads 4.2.7.2 Equivalent vertical loading for new earthworks and earth pressure effects imposed on new structures 4.2.7.4 Resistance of existing bridges and earthworks to traffic loads	4.2.3.2 Compatibility with load carrying capacity of lines
Running dynamic behaviour	4.2.8 Immediate action limits on track geometry defects	4.2.3.5.2 Running dynamic behaviour
Longitudinal actions	4.2.6.2 Longitudinal track resistance 4.2.7.1.5 Actions due to traction and braking (longitudinal loads)	4.2.4.3.2 Brake performance
Minimum curve radius	4.2.3.4 Minimum radius of horizontal curve	4.2.2.1. Mechanical interface
Vertical curve	4.2.3.5 Minimum radius of vertical curve	4.2.3.1 Gauging

4.3.2 Interfaces with the energy subsystem

States shall ensure that the interfaces with the energy subsystem are managed.

Table 17 Interfaces with the energy subsystem

Interface	Reference in TSI INF	Reference in TSI ENE
Gauge	4.2.3.1 Structure gauge	4.2.10 Pantographs gauge

4.3.3 Interfaces with the control command and signalling subsystem

States shall ensure that the interfaces with the control command and signalling subsystem are managed.

Table 18 Interfaces with the control command and signalling subsystem

Interface	Reference in TSI INF	Reference in TSI CCS
Structure gauge set for CCS installations. Visibility of track- side CCS objects.	4.2.3.1 Structure gauge	4.2.5.2 Eurobalise communication (space for installation) 4.2.5.3 Euroloop communication (space for installation) 4.2.10 Train detection systems (space for installation) 4.2.15 Visibility of track-side control-command and signalling objects

4.3.4 Interfaces with the operation and traffic management subsystem

States shall ensure that in accordance with UTP specifications, as far as these are available, operational measures are in place to manage the following interfaces between the infrastructure and the operation of trains:

- Ride stability with a view to the in-service equivalent conicity
- Use of eddy current brakes with a view to longitudinal track resistance and braking performance of trains
- Limiting the effect of crosswinds
- Operating rules
- Staff competences.

Table 19 Interfaces with the operation and traffic management subsystem

Interface	Reference in TSI INF	Reference in TSI OPE
Ride stability	4.2.11.2 Equivalent conicity in service	4.2.3.4.4. Operational quality
Use of eddy current brakes	4.2.6.2 Longitudinal track resistance	4.2.2.6.2 Braking performance
Crosswinds	4.2.10.2 Effect of crosswinds	4.2.3.6.3 Contingency arrangements
Operating rules	4.4 Operating rules	4.2.1.2.2.2 Modifications to information

		contained in the route book 4.2.3.6 Degraded operation
Staff competences	4.6 Professional competences	4.2.1.1 General requirements

4.4 Operating rules

(Reserved)

(1) Operating rules are developed within the procedures described in the infrastructure manager's safety management system. These rules take into account the documentation related to operation which forms a part of the technical file as required in Article 15(4) and set out in Annex IV (point 2.4) of Directive (EU) 2016/797.

(2) In certain situations involving pre-planned works, it may be necessary to temporarily suspend the specifications of the infrastructure subsystem and its interoperability constituents defined in sections 4 and 5 of this TSI.

4.5 Maintenance rules

(Reserved)

(1) Maintenance rules are developed within the procedures described in the infrastructure manager's safety management system.

(2) The maintenance file shall be prepared before placing a line into service as the part of the technical file accompanying the TSI declaration of verification.

(3) The maintenance plan shall be drawn up for the subsystem to ensure that the requirements set out in this TSI are maintained during its lifetime.

4.5.1 Maintenance file

Contracting States shall ensure that the entity responsible for the maintenance of infrastructure applies at least:

a) a defined set of values for immediate action limits, related to track geometric quality and limits on isolated defects;

b) pre-defined measures to be taken when the defined limits are not met (such measures could for example involve speed restriction and repair time).

A maintenance file shall contain at least:

a) a set of values for immediate action limits,

b) the measures taken (for example speed restriction, repair time) when prescribed limits are not met,

related to track geometric quality and limits on isolated defects.

4.5.2 Maintenance plan

Contracting States shall ensure that the entity responsible for the maintenance of infrastructure has a maintenance plan

The infrastructure manager shall have a maintenance plan

containing the items listed in point 4.5.1 together with at least the following:

- a set of values for intervention limits and alert limits,
- a statement about the methods, professional competences of staff and personal protective safety equipment necessary to be used,
- the rules to be applied for the protection of people working on or near the track,
- the means used to check that in-service values are respected,
- the measures taken, for speed greater than 250 km/h, to mitigate the risk of ballast pick up.

4.6 Professional qualifications

(Reserved)

The professional qualifications of staff required for operation and maintenance of the infrastructure subsystem are not set out in this TSI but are described in the infrastructure manager's safety management system.

4.7 Health and safety conditions

(Reserved)

(1) The health and safety conditions of staff required for the operation and maintenance of the infrastructure subsystem shall be compliant with the relevant European and national legislation.

(2) The issue is covered by the procedures described in the infrastructure manager's safety management system.

5. INTEROPERABILITY CONSTITUENTS

5.1 Basis on which interoperability constituents have been selected

(1) The requirements of point 5.3 are based on a traditional design of ballasted track with Vignole (flat-bottom) rail on concrete or wooden sleepers and fastening providing resistance to longitudinal slip by bearing on the rail foot.

(2) Components and subassemblies used for the construction of other designs of track are not considered to be interoperability constituents.

5.2 List of constituents

(1) For the purposes of this UTP, only the following elements, whether individual components or subassemblies of the track are declared to be "interoperability constituents[13]":

a) the rail (5.3.1);

b) the rail fastening systems (5.3.2);

c) track sleepers (5.3.3).

(2) The following points describe the specifications applicable to each of these constituents.

(3) Rails, fastenings and sleepers used for short length of track for specific purposes, for example in switches and crossings, at expansion devices, transition slabs and special structures, are not considered to be interoperability constituents.

5.3 Constituents performances and specifications

5.3.1 The rail

The specifications of the "rail" interoperability constituent concern the following parameters:

a) railhead profile;

b) rail steel.

5.3.1.1 Railhead profile

The rail head profile shall fulfil the requirements of point 4.2.4.6 "Railhead profile for plain line".

5.3.1.2 Rail steel

(1) The rail steel is relevant to the requirements of point 4.2.6 "Track resistance to applied loads".

(2) The rail steel shall meet the following requirements:

a) The rail hardness shall be at least 200 HBW;

b) The tensile strength shall be at least 680 MPa;

c) Minimum number of cycles at fatigue test without failure shall be at least 5×10^6.

5.3.2 The rail fastening systems

(1) The rail fastening system is relevant to the requirements of point 4.2.6.1 for "Track resistance to vertical loads", point 4.2.6.2 for "Longitudinal track resistance" and point 4.2.6.3 for "Lateral track resistance".

[13] Interoperability Constituents are defined in Article 2(g) of ATMF. The separate assessment of ICs is not mandatory in COTIF. In case the assessment of conformity of an IC is not done separately, it should take place together with the assessment of the subsystem.

(2) The rail fastening system shall comply in laboratory test conditions with the following requirements:

a) the longitudinal force required to cause the rail to begin to slip (i.e. move in an inelastic way) through a single rail fastening assembly shall be at least 7 kN and for speeds of more than 250 km/h shall be at least 9 kN,

b) the rail fastening shall resist application of 3 000 000 cycles of the typical load applied in a sharp curve, such that the change in performance of the fastening system shall not exceed:

- 20 % in terms of clamping force,
- 25 % in terms of vertical stiffness,
- a reduction of more than 20 % in terms of longitudinal restraint.

The typical load shall be appropriate to:

- the maximum axle load the rail fastening system is designed to accommodate,
- the combination of rail, rail inclination, rail pad and type of sleepers with which the fastening system may be used.

5.3.3 Track sleepers

(1) Track sleepers shall be designed such that when they are used with a specified rail and rail fastening system they will have properties that are consistent with the requirements of point 4.2.4.1 for "Nominal track gauge", point 4.2.4.7 for "Rail inclination" and point 4.2.6 for "Track resistance to applied loads".

(2) For the nominal track gauge system of 1 435 mm, the design track gauge for track sleepers in straight alignments and in horizontal curves with radius greater than 300 m shall be 1 437 mm.

6. ASSESSMENT OF CONFORMITY OF INTEROPERABILITY CONSTITUENTS AND OF THE SUBSYSTEM[14]

The admission of infrastructure is subject to the provisions in force in the state in which the infrastructure is located (see Article 8 § 2 of the ATMF UR).

Conformity assessment responsibilities and procedures, including declarations, are therefore excluded from this UTP.

Contracting States are recommended to have mechanisms and procedures in place which promote and enable robust and reliable conformity assessment. These include provisions such that assessments are made only by persons that possess adequate qualifications and are sufficiently independent. For this purpose, Contracting States are recommended to apply criteria similar to those defined in UTP GEN-E.

Modules for the procedures for assessment of conformity, suitability for use and EC verification are defined in Article 8 of this Regulation[15].

[14] The title in the INF TSI reads: "6. Assessment of conformity of interoperability constituents and EC verification of the subsystems"

[15] Article 8 of the enacting part of the INF TSI.

6.1 Interoperability constituents

6.1.1 Conformity assessment procedures

Unless specified otherwise, the assessment of conformity is subject to the rules applicable in the state concerned.

(1) The conformity assessment procedure of interoperability constituents as defined in section 5 of this TSI shall be carried out by application of the relevant modules.

(2) Serviceable interoperability constituents that are suitable for reuse are not subject to the conformity assessment procedures.

6.1.2 Application of modules

(Reserved)

(1) The following modules for conformity assessment of interoperability constituents are used:

a) CA 'Internal production control'

b) CB 'Type examination'

c) CC 'Conformity to type based on internal production control'

d) CD 'Conformity to type based on quality management system of the production process'

e) CF 'Conformity to type based on product verification'

f) CH 'Conformity based on full quality management system'

(2) The modules for conformity assessment of interoperability constituents shall be chosen from those shown in Table 20.

Table 20 Modules for conformity assessment to be applied for interoperability constituents

Procedures	Rail	Rail fastening system	Track sleepers
Placed on the EU market before entry into force of relevant TSIs	CA or CH	CA or CH	

Placed on the EU market after entry into force of relevant TSIs	CB + CC or CB + CD or CB + CF or CH	

(3) In the case of products placed on the market before the publication of relevant TSIs, the type is considered to have been approved and therefore EC type examination (module CB) is not necessary, provided that the manufacturer demonstrates that tests and verification of interoperability constituents have been considered successful for previous applications under comparable conditions and are in conformity with the requirements of this TSI. In this case these assessments shall remain valid in the new application. If it is not possible to demonstrate that the solution is positively proven in the past, the procedure for interoperability constituents placed on the EU market after publication of this TSI applies.

(4) The conformity assessment of interoperability constituents shall cover the phases and characteristics as indicated in Table 36 of Appendix A to this TSI.

6.1.3 Innovative solutions for interoperability constituents

If an innovative solution is proposed for an interoperability constituent, the procedure described below shall apply:

In order to keep pace with technological progress, innovative solutions may be required, which do not comply with the specifications set out in this UTP. In that case, new specifications associated with those innovative solutions shall be developed.

Innovative solutions may be related to the infrastructure subsystem, its parts and its ICs.

If an innovative solution is proposed, the manufacturer or his authorised representative shall declare how it deviates from or complements the relevant provisions of this UTP and submit the deviations to the Competent Authority of the State where the infrastructure is located. If the Competent Authority is of the opinion that the UTP should be

in Article 10[16] shall apply.

Article 10, Innovative solutions

1. In order to keep pace with technological progress, innovative solutions may be required, which do not comply with the specifications set out in the Annex or for which the assessment methods set out in the Annex cannot be applied.

2. Innovative solutions may relate to the infrastructure subsystem, its parts and its interoperability constituents.

3. If an innovative solution is proposed, the manufacturer or his authorised representative established within the Union shall declare how it deviates from or complements to the relevant provisions of this TSI and submit the deviations to the Commission for analysis. The Commission may

[16] Article 10 of the enacting part of the INF TSI.

modified in order to take the innovative solution into account, it shall submit its proposal to the Committee of Technical Experts (CTE). If the CTE supports the opinion, the appropriate functional and interface specifications, which need to be included in the UTP in order to allow the use of this innovative solution, shall be developed and subsequently integrated in the UTP during its revision processes.	*request the opinion of the Agency on the proposed innovative solution.* *4. The Commission shall deliver an opinion on the proposed innovative solution. If this opinion is positive, the appropriate functional and interface specifications and the assessment method, which need to be included in the TSI in order to allow the use of this innovative solution, shall be developed and subsequently integrated in the TSI during the revision process pursuant to Article 5 of Directive* (EU) 2016/797. *If the opinion is negative, the innovative solution proposed cannot be used.* *5. Pending the review of the TSI, the positive opinion delivered by the Commission shall be considered as an acceptable means of compliance with the essential requirements of Directive* (EU) 2016/797 *and may be used for the assessment of the subsystem.*

6.1.4 Declaration of conformity for interoperability constituents

6.1.4.1 Interoperability constituents subject to other European Union Directives

(Reserved)	(1) in accordance with Article 10 (3) of Directive (EU) 2016/797, for interoperability constituents that are the subject of other legal acts of the Union covering other matters, the EC declaration of conformity or suitability for use shall state that the interoperability constituents also meet the requirements of those other legal acts; (2) in accordance with Annex I to Commission Implementing Regulation (EU) 2019/250[17], the EC declaration of conformity or suitability for use shall include a list of restrictions or condition of use.

6.1.4.2 Declaration of conformity for rails

(Reserved)	No statement setting out the conditions of use is required.

6.1.4.3 Declaration of conformity for rail fastening systems

(Reserved)	The declaration of conformity shall be accompanied by statement setting out:

[17] Commission Implementing Regulation (EU) 2019/250 of 12 February 2019 on the templates for "EC" declarations and certificates for railway interoperability constituents and subsystems, on the model of declaration of conformity to an authorised railway vehicle type and on the "EC" verification procedures for subsystems in accordance with Directive (EU) 2016/797.

a) the combination of rail, rail inclination, rail pad and type of sleepers with which the fastening system may be used,

b) the maximum axle load the rail fastening system is designed to accommodate.

6.1.4.4 Declaration of conformity for track sleepers

(Reserved)

The declaration of conformity shall be accompanied by statement setting out:

a) the combination of rail, rail inclination and type of rail fastening system with which the sleeper may be used,

b) the nominal and design track gauge,

c) the combinations of axle load and train speed the track sleeper is designed to accommodate.

6.1.5 Particular assessment procedures for interoperability constituents

Particular assessment procedures, described in point 6.1.5.1 below, fall within the scope of this UTP. These procedures are necessary to ensure that conformity assessment of parameters in this UTP is carried out in a harmonised manner.

6.1.5.1 Assessment of rails

Assessment of rail steel shall be done according to the following requirements:

a) Rail hardness shall be tested for position RS in accordance with the specification referenced in Appendix T, Index [7].

b) Tensile strength shall be tested in accordance with the specification referenced in Appendix T, Index [7].

c) Fatigue test shall be done in accordance with the specification referenced in Appendix T, Index [7].

6.1.5.2 Assessment of sleepers

(Reserved)

(1) (not used)

(2) For polyvalent gauge and multiple gauge track sleepers it is allowed not to assess the design track gauge for the nominal track gauge of 1 435 mm.

6.2 Infrastructure subsystem

6.2.1 General provisions

Unless specified otherwise, the assessment of conformity is subject to the rules applicable in the state concerned.

(1) At the request of the applicant, the notified body carries out the EC verification of the infrastructure subsystem in accordance with Article 15 of Directive (EU) 2016/797 and in accordance with the provisions of the relevant modules.

(2) If the applicant demonstrates that tests or assessments of an infrastructure subsystem or parts of the subsystem are the same as have been successful for previous applications of a design, the notified body shall consider the results of these tests and assessments for the EC verification.

(3) The EC verification of the infrastructure subsystem shall cover the phases and characteristics indicated in Table 37 in Appendix B to this TSI.

(4) Performance parameters as set out in point 4.2.1 of this TSI are not subject to the EC verification of the subsystem.

(5) Particular assessment procedures for specific basic parameters of infrastructure subsystem are set out in point 6.2.4.

(6) The applicant shall draw up the EC declaration of verification for the infrastructure subsystem in accordance with Article 15 of Directive (EU) 2016/797.

6.2.2 Application of modules

(Reserved)

For the EC verification procedure of the infrastructure subsystem, the applicant may choose either:

a) Module SG: EC verification based on unit verification, or

b) Module SH1: EC verification based on full quality management system plus design examination.

6.2.2.1. Application of module SG

In the case where EC verification is most effectively undertaken by using information collected by the infrastructure manager, contracting entity or the main contractors

	involved (for example data obtained using track recording vehicle or other measuring devices), the notified body shall take this information into account to assess conformity. 6.2.2.2. Application of module SH1 The SH1 module may be chosen only where the activities contributing to the proposed subsystem to be verified (design, manufacturing, assembling, installation) are subject to a quality management system for design, production, final product inspection and testing, approved and surveyed by a notified body.

6.2.3 Innovative solutions

If an innovative solution is proposed for the infrastructure subsystem, the procedure described in point 6.1.3 shall apply.	Article 10[18] shall apply.

6.2.4 Particular assessment procedures for infrastructure subsystem

Particular assessment procedures, as described under points 6.2.4.1 to 6.2.4.12, fall within the scope of this UTP. These procedures are necessary to ensure that conformity assessment of parameters in this UTP is carried out in a harmonised manner.	

6.2.4.1 Assessment of Structure gauge

(1) Assessment of structure gauge as a design review shall be done against characteristic cross sections using the results of calculations made by infrastructure manager or the contracting entity on the basis of the specification referenced in Appendix T, Index [3].

(2) Characteristic cross sections are:

a) track without cant;

b) track with maximum cant;

c) track with a civil engineering structure over the line;

d) any other location where the designed installation limit gauge is approached by less than 100 mm or the installation nominal gauge or uniform gauge is approached by less than 50 mm.

(3) After assembly before putting into service clearances shall be verified at locations where the designed installation limit gauge is approached by less than 100 mm or the installation nominal gauge or uniform gauge is approached by less than 50 mm.

[18] Article 10 of the enacting part of the INF TSI.

(4) Instead of point (1), for the 1 520 mm track gauge system assessment of structure gauge as a design review is to be made against characteristic cross sections using the uniform structure gauge "S" as defined in Appendix H.

(5) Instead of point (1), for the 1 600 mm track gauge system assessment of structure gauge as a design review is to be made against characteristic cross sections using the structure gauge "IRL1" as defined in Appendix O.

6.2.4.2 Assessment of distance between track centres

(1) A design review for assessment of the distance between track centres shall be done using the results of calculations made by the Infrastructure Manager or the contracting entity on the basis of the specification referenced in Appendix T, Index [3]. The nominal distance between track centres shall be checked at the line layout where distances are given in parallel to the horizontal plane. The limit installation distance between track centres shall be checked with the radius and relevant cant.

(2) After assembly before putting into service, distance between track centres shall be verified at critical locations where the limit installation distance between track centres as defined in accordance with the specification referenced in Appendix T, Index [3] is approached by less than 50 mm.

(3) Instead of point (1), for the 1 520 mm track gauge system a design review for assessment of the distance between track centres is to be made using the results of calculations made by the infrastructure manager or the contracting entity. The nominal distance between track centres shall be checked at the line layout where distances are given in parallel to the horizontal plane. The limit installation distance between track centres shall be checked with the radius and relevant cant.

(4) Instead of point (2), for the 1 520 mm track gauge system after assembly before putting into service, distance between track centres shall be verified at critical locations where the limit installation distance between track centres is approached by less than 50 mm.

6.2.4.3 Assessment of nominal track gauge

(1) (Reserved)	Assessment of the nominal track gauge at design review shall be done by checking the self-declaration of the applicant.
(2) (Reserved)	Assessment of the nominal track gauge at assembly before putting into service shall be done by checking the interoperability constituent sleeper's certificate. For non-certified interoperability constituents assessment of the nominal track gauge shall be done by checking the self-declaration of the applicant.

6.2.4.4 Assessment of track layout

(1) At design review the curvature, cant, cant deficiency and abrupt change of cant deficiency shall be assessed against the local design speed.

(2) Assessment of switches and crossings layout is not required.

(3) At assembly before putting into service, for the review of the minimum horizontal curve the measurement values provided by the applicant or infrastructure manager shall be assessed. Rules for acceptance of works defined by the infrastructure manager shall be taken into account.

6.2.4.5 Assessment of cant deficiency for trains designed to travel with higher cant deficiency

Point 4.2.4.3(2) states that "It is permissible for trains specifically designed to travel with higher cant deficiency (for example multiple units with lower axle loads; vehicles with special equipment for the negotiation of curves) to run with higher cant deficiency values, subject to a demonstration that this can be achieved safely".

This demonstration is outside the scope of this TSI and thus not subject to a notified body verification of the infrastructure subsystem. The demonstration shall be undertaken by the RU, if necessary in cooperation with the IM.

6.2.4.6 Assessment of design values for equivalent conicity

Assessment of design values for equivalent conicity shall be done using the results of calculations made by the infrastructure manager or the contracting entity on the basis of the specification referenced in Appendix T, Index [5].

6.2.4.7 Assessment of railhead profile

(1) The design profile of new rails shall be checked against point 4.2.4.6.

(2) Reused serviceable rails shall not be subject to the requirements for railhead profile as set out in point 4.2.4.6.

6.2.4.8 Assessment of switches and crossings

(Reserved)

Assessment of switches and crossings related to points 4.2.5.1 to 4.2.5.3 shall be done by checking that a self-declaration of the infrastructure manager or contracting entity exists.

6.2.4.9 Assessment of new structures, earthworks and earth pressure effects

(1) Assessment of new structures shall be done by checking the traffic loads and the track twist limit used for design against the minimum requirements of points 4.2.7.1 and 4.2.7.3.

The notified body is not required to review the design nor carry out any calculations.

When reviewing the value of factor alpha used in the design according to point 4.2.7.1 it is only necessary to check that the value of factor alpha satisfies Table 11.

(2) Assessment of new earthworks and earth pressure effects shall be done by checking the vertical loads used for design according to requirements of point 4.2.7.2. When reviewing the value of factor alpha used in the design according to point 4.2.7.2 it is only necessary to check that the value of factor alpha satisfies Table 11.

	The notified body is not required to review the design nor carry out any calculations.

6.2.4.10 Assessment procedure of existing structures

(1) The assessment of existing structures against the requirements of point 4.2.7.4(3) (b) and (c) shall be done by one of the following methods:

a) A check that the values of EN line categories, in combination with the allowed speed published, or intended to be published, for the lines containing the structures, are in line with the requirements of Appendix E;

b) A check that the values of EN line categories, in combination with the allowed speed specified for the bridges or for the design, or alternative requirements specified with LM71 and factor alpha (α) for P1 and P2, are in line with the requirements of Appendix E;

c) A check that the traffic loads specified for the structures or for the design against the minimum requirements of points 4.2.7.1.1, 4.2.7.1.2 and 4.2.7.2. When reviewing the value of factor alpha (α) in accordance with points 4.2.7.1.1 and 4.2.7.2, it is only necessary to check that the value of factor alpha (α) is in line with the value of factor alpha (α) set out in Table 11;

d) Where the requirement for an existing bridge is specified by reference to the design load model HSLM in Appendix E, the assessment of the existing bridge shall be done by either of the following methods:

- checking the specification of the design of the existing bridge,
- checking the specification of the dynamic appraisal,
- checking the published load carrying capacity of the existing bridge

provided by the infrastructure manager in accordance with the UTP TCRC;	in the register of infrastructure (RINF) for the parameter 1.1.1.1.2.4.2 (Compliance of structures with the High Speed Load Model (HSLM));

e) Where the requirement for an existing bridge is specified by reference to alternative dynamic loading requirements (Appendix E note 8), the assessment of the existing bridge shall be done by checking the specification of the dynamic appraisal for these alternative loading requirements against the requirements in Appendix E note 8.

(2) It is not required to review the design nor carry out any calculations.

(3) For existing structures assessment point 4.2.7.4(4) applies respectively.

6.2.4.11 Assessment of platform offset

(1) Assessment of the distance between the track centre and the platform edge as a design review shall be done using the results of calculations made by the Infrastructure Manager or the contracting entity on the basis of the specification referenced in Appendix T, Index [3].

(2) After assembly before putting into service clearances shall be verified. The offset is checked at the ends of the platform and every 30 m in straight track and every 10 m in curved track.

(3) Instead of point (1), for the 1 520 mm track gauge system assessment of the distance between the track centre and the platform edge as a design review shall be done against requirements of point 4.2.9.3. Point (2) applies accordingly.

(4) Instead of point (1), for the 1 600 mm track gauge system assessment of the distance between the track centre and the platform edge as a design review shall be done against requirements of point 4.2.9.3(4). Point (2) applies accordingly.

6.2.4.12 Assessment of maximum pressure variations in tunnels

(1) The assessment of the maximum pressure variation in the tunnel (10 kPa criterion) shall be done in accordance with the specification referenced in Appendix T, Index [14] with trains that comply with the UTP LOC&PAS and that are able to run at maximum line speed in the specific tunnel to be assessed.

(2) The input parameters to be used during the assessment shall be such that the reference characteristic pressure signature of the trains set out in the UTP LOC&PAS is fulfilled.

(3) The reference cross section areas are set out in the specification referenced in Appendix T, Index [14].

6.2.4.13 Assessment of effect of crosswinds

(Reserved)	This demonstration of the safety is outside the scope of this TSI and thus not subject to a notified body verification. The demonstration shall be undertaken by the infrastructure manager, if necessary in cooperation with the railway undertaking.

6.2.4.14 Assessment of fixed installations for servicing trains

(Reserved)	Assessment of fixed installations for servicing trains is in the responsibility of the Member State concerned.

6.2.4.15 Assessment of compatibility with braking systems

The assessment of the requirements laid down in point 4.2.6.2.2(2) is not required.

6.2.5 Technical solutions giving presumption of conformity at design stage

The admission of infrastructure is not in the scope of COTIF and this UTP should not therefore stipulate binding provisions concerning the responsibilities and procedures for conformity assessment. The provisions in point 6.2.5 are therefore recommended best practices.	Presumption of conformity at design stage for technical solutions may be assessed prior and independent from a specific project.

6.2.5.1 Assessment of track resistance for plain line

(1) The demonstration of conformity of the track to the requirements of point 4.2.6 may be done by reference to an existing track design which meets the operating conditions intended for the subsystem concerned.

(2) A track design shall be defined by the technical characteristics as set out in Appendix C.1 and by its operating conditions as set out in Appendix D.1.

(3) A track design is considered to be existing, if both of the following conditions are met:

a) the track design has been in normal operation for at least one year; and

b) the total tonnage over the track was at least 20 million gross tons for the period of normal operation.

(4) The operating conditions for an existing track design refer to conditions which have been applied in normal operation.

(5) The assessment to confirm an existing track design shall be performed by checking that the technical characteristics as set out in Appendix C.1 and conditions of use as set out in Appendix D.1 are specified and that the reference to the previous use of the track design is available.

(6) When a previously assessed existing track design is used in a project,

the conditions of use should be the same.	the notified body shall only assess that the conditions of use are respected.

(7) For new track designs that are based on existing track designs, a new assessment can be performed by verifying the differences and evaluating their impact on the track resistance. This assessment may be supported for example by computer simulation or by laboratory or in situ testing.

(8) A track design is considered to be new, if at least one of the technical characteristics set out in Appendix C or one of conditions of use set out in Appendix D is changed.

6.2.5.2 Assessment for switches and crossing

(1) The provisions as set out in point 6.2.5.1 are applicable for the assessment of track resistance for switches and crossings. Appendix C.2 sets out the technical characteristics of switches and crossings design and Appendix D.2 sets out the conditions of use of switches and crossings design.

(2) Assessment of design geometry of switches and crossings shall be done according to point 6.2.4.8.

(3) Assessment of maximum unguided length of fixed obtuse crossings shall be done according to point 6.2.4.8.

6.3 (Reserved)

6.4 Assessment of maintenance file

(1) (Reserved)

In accordance with Article 15(4) of Directive (EU) 2016/797 of the European Parliament and of the Council[19], the applicant shall be responsible for compiling the technical file, containing the documentation requested for maintenance.

(2) (Reserved)

The notified body shall verify only that the documentation requested for maintenance, as set out in point 4.5.1, is provided. The notified body is not required to verify the information contained in the documentation provided.

6.5 Subsystems containing interoperability constituents not holding an EC declaration

6.5.1 Conditions

(1) (Reserved)

Until the list of interoperability constituents listed in Chapter 5 of this TSI are revised, a notified body is allowed to issue an EC certificate of verification for a subsystem even if some of the interoperability constituents incorporated within the subsystem are not covered by the relevant EC declarations of conformity and/or suitability for use according to this TSI, if the following criteria are complied with:

a) the conformity of the subsystem has been checked against the requirements of section 4 and in relation to sections 6.2 to 7 (except point 7.7 'Specific Cases') of this TSI by the notified body. Furthermore the conformity of the ICs to section 5 and 6.1 does not apply, and

b) the interoperability constituents, which are not covered by the relevant EC declaration of conformity and/or suitability for use, have been used in a subsystem already

[19] Directive (EU) 2016/797 of the European Parliament and of the Council of 11 May 2016 on the interoperability of the rail system within the European Union.

	approved and put in service in at least one of the Member State before the entry in force of this TSI.
(2) (Reserved)	EC Declarations of conformity and/or suitability for use shall not be drawn up for the interoperability constituents assessed in this manner.

6.5.2 Documentation

(1) (Reserved)	The EC certificate of verification of the subsystem shall indicate clearly which interoperability constituents have been assessed by the notified body as part of the subsystem verification.
(2) (Reserved)	The EC declaration of verification of the subsystem shall indicate clearly: a) Which interoperability constituents have been assessed as part of the subsystem; b) Confirmation that the subsystem contains the interoperability constituents identical to those verified as part of the subsystem; c) For those interoperability constituents, the reason(s) why the manufacturer did not provide an EC Declaration of conformity and/or suitability for use before its incorporation into the subsystem, including the application of national rules notified under Article 14 of Directive (EU) 2016/797.

6.5.3 Maintenance of the subsystems certified according to 6.5.1.

(1) (Reserved)	During and after the transition period and until the subsystem is upgraded or renewed (taking into account the decision of Member State on application of TSIs), the interoperability constituents which do not hold an EC Declaration of conformity and/or suitability for use and are of the same type are allowed to be used as maintenance related replacements (spare parts) for the subsystem, under the responsibility of the body responsible for maintenance.
(2) (Reserved)	In any case the body responsible for maintenance must ensure that the components

for maintenance related replacements are suitable for their applications, are used within their area of use and enable interoperability to be achieved within the rail system while at the same time meeting the essential requirements. Such components must be traceable and certified in accordance with any national or international rule or any code of practice widely acknowledged in the railway domain.

6.6 Subsystem containing serviceable interoperability constituents that are suitable for reuse

6.6.1 Conditions

(1) (Reserved)

A notified body is allowed to issue an EC certificate of verification for a subsystem even if some of the interoperability constituents incorporated within the subsystem are serviceable interoperability constituents that are suitable for reuse, if the following criteria are complied with:

a) the conformity of the subsystem has been checked against the requirements of section 4 and in relation to sections 6.2 to 7 (except point 7.7 “Specific Cases”) of this TSI by the notified body. Furthermore the conformity of the ICs to 6.1 does not apply, and

b) the interoperability constituents are not covered by the relevant EC declaration of conformity and/or suitability for use.

(2) (Reserved)

EC declarations of conformity and/or suitability for use shall not be drawn up for the interoperability constituents assessed in this manner.

6.6.2 Documentation

(1) (Reserved)

The EC certificate of verification of the subsystem shall indicate clearly which interoperability constituents have been assessed by the notified body as part of the subsystem verification.

(2) (Reserved)

The EC declaration of verification of the subsystem shall indicate clearly:

a) Which interoperability constituents are serviceable interoperability constituents that are suitable for reuse;

b) Confirmation that the subsystem contains the interoperability constituents identical to those verified as part of the subsystem.

6.6.3 Use of serviceable interoperability constituents in maintenance

(1) (Reserved)

Serviceable interoperability constituents that are suitable for reuse are allowed to be used as maintenance related replacements (spare parts) for the subsystem, under the responsibility of the body responsible for maintenance.

(2) (Reserved)

In any case the body responsible for maintenance must ensure that the components for maintenance related replacements are suitable for their applications, are used within their area of use, and enable interoperability to be achieved within the rail system while at the same time meeting the essential requirements. Such components must be traceable and certified in accordance with any national or international rule, or any code of practice widely acknowledged in the railway domain.

7. IMPLEMENTATION OF THE UTP INFRASTRUCTURE

See point 1.2. for the scope of application.

Contracting States shall publish a list of lines to which this UTP is applicable, indicating for each line whether it fully complies with the technical provisions of this UTP. Non-compliance shall be identified, where possible indicating the characteristics of the line which deviate from the UTP provisions and where these deviations occur. Stations where lines with different characteristics come together shall be indicated.

7.1 National implementation plan

(Reserved)

Member States shall develop a national plan for the implementation of this TSI, targeting the coherence of the entire rail system of the Union. This plan shall include all projects regarding new, renewal and upgrading of infrastructure subsystem and shall ensure a

gradual migration within a reasonable timescale onwards an interoperable target infrastructure subsystem fully compliant with this TSI.

7.2 Application of this UTP to new infrastructure subsystem

(1) (Reserved)

For a new infrastructure subsystem, the application of this TSI shall be compulsory.

(2) (Reserved)

A 'new infrastructure subsystem' means an infrastructure subsystem placed into service after 28 September 2023 which creates a route or a part of a route where none currently exists.

Any other infrastructure subsystems shall be considered as 'existing infrastructure subsystems'.

(3) (Reserved)

The following cases are considered as upgrading and not as the placing into service of a new infrastructure subsystem:

a) the realignment of part of an existing route,

b) the creation of a bypass,

c) the addition of one or more tracks on an existing route, regardless of the distance between the original tracks and the additional tracks.

7.3 Application of this UTP to existing infrastructure subsystem

7.3.1 Performance criteria of the subsystem

In addition to the cases referred to in point 7.2 (3),

"Upgrading" is a major modification work to an existing infrastructure subsystem resulting in at least compliance with one additional traffic code or a change in the declared combination of traffic codes (referred to in Table 2 and Table 3 in point 4.2.1).

7.3.2 Application of the UTP

The conformity with this TSI is mandatory for a subsystem or part(s) of it which are upgraded or renewed.

Due to the characteristics of the inherited railway system, compliance of existing infrastructure subsystem with this UTP may be achieved through the gradual improvement of interoperability:

(1) (Reserved)

For the upgraded infrastructure subsystem, the application of this TSI shall be compulsory, and

applied to the upgraded subsystem within the geographical coverage of the upgrading. The geographical coverage of the upgrading shall be defined based on locations on tracks and metric references and shall result in the compliance of all basic parameters of the infrastructure subsystem associated with the tracks that are subject to the upgrading of the infrastructure subsystem.

The addition of one or more rails supporting a further track gauge is also considered as upgrade when the performance criteria of the subsystem is triggered as described in point 7.3.1.

(2) (Reserved)

In the event of a change other than an upgrading of the infrastructure subsystem, the application of this TSI for each basic parameters (referred to in point 4.2.2) affected by a change shall be compulsory when the change requires to carry out a new TSI verification procedure in accordance with Implementing Regulation (EU) 2019/250[20]. Provisions defined in Articles 6 and 7 of Implementing Regulation (EU) 2019/250 shall apply.

(3) In the event of a change other than an upgrade of the infrastructure subsystem and for those basic parameters that are not affected by the change, or when the change does not require a new UTP verification procedure, the demonstration of the level of compliance with this UTP is voluntary.

(4) In case of upgrading or renewal of the infrastructure subsystem, the compliance with the requirements which are laid down for new lines is not required.

(5) (Reserved)

In case of "major substitution", as defined in Article 2(15) of Directive (EU) 2016/797 of the European Parliament and of the Council, in the framework of a "renewal", non TSI-compliant elements of the subsystem or part(s) of it shall systematically be replaced with TSI-compliant ones.

(6) "Substitution in the framework of maintenance" means any replacement of components by parts of identical function and performance in the framework of maintenance

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, as defined in Article 2(17) of Directive (EU) 2016/797. It shall be made in accordance with the requirements of this TSI, whenever

[20] Commission Implementing Regulation (EU) 2019/250 of 12 February 2019 on the templates for 'EC' declarations and certificates for railway interoperability constituents and subsystems, on the model of declaration of conformity to an authorised railway vehicle type and on the 'EC' verification procedures for subsystems in accordance with Directive (EU) 2016/797 of the European Parliament and of the Council.

	reasonably and economically feasible and it does not require a TSI verification procedure.
(7) (Reserved)	The following exceptions are permitted for existing infrastructure subsystem, in case of upgrading or renewal: a) In the case of upgrading or renewal of the infrastructure subsystem, for parameters cant governed by point 4.2.4.2 of this TSI and cant deficiency governed by point 4.2.4.3 of this TSI it is permitted to deviate from the limiting values as set out in this TSI while respecting the exceptional limit values and applying specific restrictions and measures set out in the specification referenced in Appendix T, index [4]. Applying this exception shall not prevent the access of vehicles admitted to international traffic for the maximum values required in point 4.2.4.3 of this TSI. b) In the event of a change other than an upgrading of the infrastructure subsystem, the following conditions related to platform height and offset governed by points 4.2.9.2 and 4.2.9.3, shall apply: – It shall be allowed to apply other nominal platform heights, if the compliance to the values set out by point 4.2.9.2 would require structural alterations to any load bearing element. – It shall be allowed to apply other platform offset than the one defined in point 4.2.9.3(2) as long as the value for b_q is equal or greater than b_{qlim}.

7.3.3 Existing lines that are not subject to a renewal or upgrading project

Demonstration of the level of compliance of existing lines opened for international traffic with the basic parameters of the UTP is voluntary.	Where an infrastructure manager wishes to demonstrate the level of compliance of an existing line with the basic parameters of this TSI, it shall apply the procedure described in

	Commission Recommendation 2014/881/EU of 18 November 2014[21].

7.3.4 Route compatibility checks before the use of authorised vehicles

The route compatibility check procedure to be applied and the parameters of the infrastructure subsystem to be used are set out in

UTP TCRC concerning train composition and route compatibility checks.	point 4.2.2.5 and Appendix D.1 of TSI OPE.

7.4 (Reserved)

7.5 (Reserved)

7.6 (Reserved)

7.7 Specific cases

The following specific cases may be applied on particular networks.

The specific cases for Member States of the European Union, or Contracting States which apply European Union law are those which are included in the INF TSI[22]. Specific cases for Switzerland are those applicable for the INF TSI as set out in Annex I of the Land Transport Agreement between the European Community and the Swiss Confederation on the Carriage of Goods and Passengers by Rail and Road, as published in the Official Journal of the European Union (OJ L 114 30.4.2002, p. 91).[23] These specific cases are not reproduced in this UTP. The specific cases are classified as: a) "P" cases: permanent cases; b) "T" cases: temporary cases. Specific cases for other Contacting States which are not members of the European Union are as follows:	The specific cases are classified as: (a) 'P' cases : permanent cases; (b) 'T' cases : temporary cases, where it is recommended that the target system is reached by 2020 (an objective set out in Decision No 1692/96/EC of the European Parliament and Council). All specific cases and their relevant dates shall be re-examined in the course of future revisions of the TSI with a view to limiting their technical and geographical scope based on an assessment of their impact on safety, interoperability, cross border services, TEN-T corridors, and the practical and economic impacts of retaining or eliminating them. Special account shall be given to availability of EU funding. Specific cases shall be limited to the route or network where they are strictly necessary and taken account of through route compatibility procedures.

[21] Commission Recommendation 2014/881/EU of 18 November 2014 on the procedure for demonstrating the level of compliance of existing railway lines with the basic parameters of the technical specifications for interoperability.

[22] See Point 7.7 of the TSI INF for specific cases of EU Member States.

[23] http://data.europa.eu/eli/agree_internation/2002/309(3)/oj

7.7.1 Specific case for Norway (“P”)

Platform offset (4.2.9.3)

As set out in point 4.2.9.3(1), the distance between the track centre and the platform edge parallel to the running plane (b_q), as defined in chapter 13 of EN 15273-3:2013, shall be calculated with the following values for allowed additional overthrow (S_{kin}):

(a) on the inside of the curve: $S_{kin} = 40,5/R$;

(b) on the outside of the curve: $S_{kin} = 31,5/R$.

7.7.2 Specific cases for the United Kingdom (Great Britain) (“P”)

This section lists specific cases which apply to the Great Britain[24] network of the United Kingdom.

7.7.2.1 Categories of line (4.2.1)

(1) Where line speeds are stated in kilometres per hour [km/h] as a category or performance parameter in this UTP, it shall be allowed to translate the speed to equivalent miles per hour [mph] as in Appendix G.

(2) Instead of the column “Gauge” in Table 2 and Table 3 of point 4.2.1 (7), for the gauge of all lines except new, dedicated high speed lines of traffic code P1, it shall be allowed to use national technical rules as set out in Appendix Q.

7.7.2.2 Structure gauge (4.2.3.1)

Instead of point 4.2.3.1, for national gauges selected according to point 7.7.1.1 (2), the structure gauge shall be set according to Appendix Q.

7.7.2.3 Distance between track centres (4.2.3.2)

(1) Instead of point 4.2.3.2, the nominal distance between track centres shall be 3 400 mm on straight track and curved track with a radius of 400 m or greater.

(2) Where topographical constraints prevent a nominal distance of 3 400 mm between track centres being achieved, it is permissible to reduce the distance between track centres provided special measures are put in place to ensure a safe passing clearance between trains.

(3) Reduction in the distance between track centres shall be in accordance with the national technical rule set out in Appendix Q.

7.7.2.4 Equivalent conicity (4.2.4.5)

(1) Instead of point 4.2.4.5 (3) design values of track gauge, rail head profile and rail inclination for plain line shall be selected to ensure that the equivalent conicity limits set out in Table 32 are not exceeded.

[24] Specific cases which apply on the Northern-Ireland network of the United Kingdom are set out in the TSI, as these are harmonised with the network of the Republic of Ireland.

Table 32 Equivalent conicity design limit values

	Wheel profile	
Speed range [km/h]	**S1002, GV1/40**	**EPS**
v ≤ 60	Assessment not required	
60 < v ≤ 200	0,25	0,30
200 < v ≤ 280	0,20	0,20
v > 280	0,10	0,15

(2) Instead of point 4.2.4.5. (4) the following wheelsets shall be modelled passing over the designed track conditions (simulated by calculation according to EN 15302:2008+A1:2010):

a) S 1002 as defined in Annex C of EN 13715:2006+A1:2010 with SR1;

b) S 1002 as defined in Annex C of EN 13715:2006+A1:2010 with SR2;

c) GV 1/40 as defined in Annex B of EN 13715:2006+A1:2010 with SR1;

d) GV 1/40 as defined in Annex B of EN 13715:2006+A1:2010 with SR2;

e) EPS as defined in Annex D of EN 13715:2006+A1:2010 with SR1.

For SR1 and SR2 the following values apply:

For the 1 435 mm track gauge system SR1 = 1 420 mm and SR2 = 1 426 mm.

7.7.2.5 Maximum unguided length of fixed obtuse crossings (4.2.5.3)

Instead of point 4.2.5.3, the design value of the maximum unguided length of fixed obtuse crossing shall be in accordance with the national technical requirement set out in Appendix Q.

7.7.2.6 The immediate action limits for switches and crossings (4.2.8.6)

Instead of point 4.2.8.6 (1)(b), for the “CEN56 Vertical” design of switches and crossings, a minimum value of fixed nose protection for common crossings of 1 388 mm is allowed (measured 14 mm below the running surface, and on the theoretical reference line, at an appropriate distance back from the actual (RP) of the nose as indicated in Figure 2).

7.7.2.7 Platform height (4.2.9.2)

Instead of point 4.2.9.2, for platform height, national technical requirements as set out in Appendix Q shall be allowed.

7.7.2.8 Platform offset (4.2.9.3)

Instead of point 4.2.9.3, for platform offset, national technical requirements as set out in Appendix Q shall be allowed.

7.7.2.9 Equivalent conicity in service (4.2.11.2)

Instead of point 4.2.11.2 (2) the infrastructure manager shall measure the track gauge and the railhead profiles at the site in question at a distance of approximate 10 m. The mean equivalent conicity over 100 m shall be calculated by modelling with the wheelsets (a) — (e) mentioned in point 7.7.1.4 (2) in order to check for compliance, for the purpose of the joint investigation, with the limit equivalent conicity for the track specified in Table 14.

7.7.2.10 Assessment of structure gauge (6.2.4.1)

Instead of point 6.2.4.1, it shall be allowed to assess structure gauge in accordance with the national technical rules as set out in Appendix Q.

7.7.2.11 Assessment of distance between track centres (6.2.4.2)

Instead of point 6.2.4.2, it shall be allowed to assess distance between track centres in accordance with the national technical rules as set out in Appendix Q.

7.7.2.12 Assessment of platform offset (6.2.4.11)

Instead of point 6.2.4.11, it shall be allowed to assess platform offset in accordance with the national technical rules as set out in Appendix Q.

APPENDIX A

Assessment of interoperability constituents

Not used

APPENDIX B

Assessment of the infrastructure subsystem

Not used

APPENDIX C

Technical characteristics of track design and switches and crossings design

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APPENDIX C.1

Technical characteristics of track design

Track design shall be at least defined by the technical characteristics as follows:

a) Rail
   - Profile(s) & grades
   - Continuous welded rail or length of rails (for jointed track sections)

b) Fastening system
   - Type
   - Pad stiffness
   - Clamping force
   - Longitudinal restraint

c) Sleeper
   - Type
   - Resistance to vertical loads:
     - Concrete: design bending moments
     - Wood: compliance with the specification referenced in Appendix T, Index [15]
     - Steel: moment of inertia of cross section
   - Resistance to longitudinal and lateral loads: geometry and weight
   - Nominal and design track gauge

d) Rail inclination

e) Ballast cross sections (ballast shoulder — ballast thickness)

f) Ballast type (grading = granulometry)

g) Sleeper spacing

h) Special devices: for example sleeper anchors, third/fourth rail, …

–

APPENDIX C.2

Technical characteristics of switches and crossings design

Switches and crossings design shall be at least defined by the technical characteristics as follows:

a) Rail
   - Profile(s) & grades (switch rail, stock rail)
   - Continuous welded rail or length of rails (for jointed track sections)

b) Fastening system
   - Type
   - Pad stiffness
   - Clamping force
   - Longitudinal restraint

c) Bearer
   - Type
   - Resistance to vertical loads:
     - Concrete: design bending moments
     - Wood: compliance with the specification referenced in Appendix T, Index [15]
     - Steel: moment of inertia of cross section
   - Resistance to longitudinal and lateral loads: geometry and weight
   - Nominal track gauge

d) Rail inclination

e) Ballast cross sections (ballast shoulder — ballast thickness)

f) Ballast type (grading = granulometry)

g) Type of crossing (fixed or movable point)

h) Type of locking (switch panel, movable point of crossing)

i) Special devices: for example sleeper anchors, third/fourth rail, …

j) Generic switches and crossings drawing indicating
   - Geometrical diagram (triangle) describing the length of the turnout and the tangents at the end of the turnout
   - Main geometrical characteristics like the main radii in switch, closure and crossing panel, crossing angle
   - Sleeper spacing

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APPENDIX D

Conditions of use of track design and switches and crossings design

–

APPENDIX D.1

Conditions of use of track design

Conditions of use of track design are defined to be as follows:

a) Maximum axle load (t)

b) Maximum line speed (km/h)

c) Minimum horizontal curve radius (m)

d) Maximum cant (mm)

e) Maximum cant deficiency (mm)

–

APPENDIX D.2

Conditions of use of switches and crossings design

Conditions of use of switches and crossings design are defined to be as follows:

a) Maximum axle load (t)

b) Maximum line speed (km/h) on through route and diverging track of switches

c) Rules for curved turnouts based on generic designs, giving minimum curvatures (for through route and diverging track of switches)

–

APPENDIX E

Capability requirements for existing structures in accordance with traffic code

The minimum capability requirements for existing bridges in accordance with point 4.2.7.4(2) are set out in Table 38A and Table 39A in accordance with the traffic codes given in Table 2 and Table 3. These capability requirements are set out using the vertical loading only, as defined by the EN line category with a corresponding speed or by LM71 with the factor alpha. Additional dynamic capability requirements are expressed by the dynamic load model HSLM. The EN line category and associated speed shall be considered as a single combined quantity.

The minimum capability requirements for existing geotechnical structures and earthworks in accordance with point 4.2.7.4(2) are set out in Table 38B and Table 39B in accordance with the traffic codes given in Table 2 and Table 3.

EN line categories area function of axle load and geometrical aspects relating to the spacing of axles and are set out in the specification referenced in Appendix T, Index [2].

For continuous bridges, the case with most onerous effects between Load Model 71 (LM71) and Load Model SW/0 shall be taken into account. LM71, Load Model SW/0 and Load Model HSLM are set out in the specification referenced in Appendix T, Index [10].

Table 38A Loading capability requirements for bridges and additional requirements due to dynamic effects (1) — Passenger traffic

Traffic code	**Traffic with loco hauled trains: Passenger trains including Carriages (Coaches, Vans and Car Carriers) and Light Freight Wagons and Locomotives and Power Heads (2) (3) (4) (5) (6)**	**Traffic with Electric or Diesel Multiple Units, Power Units and Railcars (2) (4) (5)**
P1	n.a. (7)	HSLM (8) and D2 – 200 or HSLM (8) and LM71 with α = 1.0 (14)
P2	HSLM (8) and D2 – 200 or HSLM (8) and LM71 with α = 0.91 (14)	HSLM (8) and D2 – 200 or HSLM (8) and LM71 with α = 0.91 (14)
P3a (> 160 km/h)	L ≥ 4 m D2 – 100 and L < 4 m D2 – 200 (9)(10)(15)	L ≥ 4 m C2 – 100 and L < 4 m C2 – 200 (9)(15)
P3b (≤ 160 km/h)	L ≥ 4 m D2 – 100 and L < 4 m D2 – 160 (9)(11)(15)	L ≥ 4 m D2 – 100 and L < 4 m D2 – 160 (9)(15)
P4a (> 160 km/h)	L ≥ 4 m D2 – 100 and L < 4 m D2 – 200 (9)(12)(15)	L ≥ 4 m C2 – 100 and L < 4 m C2 – 200 (9)(15)
P4b (≤ 160 km/h)	L ≥ 4 m D2 – 100 and L < 4 m D2 – 160 (9)(13)(15)	L ≥ 4 m C2 – 100 and L < 4 m C2 – 160 (9)(15)
P5	C2– 120	B1 – 120
P6	a12	
P1520	Open point	
P1600	Open point	

Table 39A Loading capability requirements for bridges expressed by EN Line Category – Associated Speed (1) — Freight traffic

Traffic code	Freight trains including freight wagons, other vehicles and locomotives (2)
F1	D4 – 120
F2	D2 – 120
F3	C2 – 100
F4	B2 – 100
F1520	Open point
F1600	Open point

Notes:

(1) The indicated speed value in the table represents the maximum requirement for the line and may be lower in accordance with the requirements in point 4.2.1(12). When checking individual structures on the line, it is acceptable to take account of the local allowed speeds as also indicated in the notes (*) and (**) of Table 2 and in the note (*) of Table 3.

(2) Passenger Carriages (including Coaches, Vans, Car Carriers), Other Vehicles, Locomotives, Power Heads, Diesel and Electric Multiple Units, Power Units and Railcars are defined in the UTP LOC & PAS. Light Freight Wagons are defined as vans except that they are allowed to be conveyed in formations which are not intended to convey passengers.

(3) The requirements for structures set out using EN line categories or load model LM71 are compatible with up to two adjacent coupled locomotives and/or power heads. The requirements for structures are compatible with a maximum speed of 120 km/h for three or more adjacent coupled locomotives and/or power heads (or a train of locomotives and/or power heads) subject to the locomotives and/or power heads satisfying the corresponding limits for freight wagons.

(4) For traffic codes P2, P3 and P4, the requirements for both traffic with loco hauled trains and traffic with multiple units shall apply. For traffic code P5, the Contracting State may indicate whether the requirements for locomotives and power heads apply.

(5) The requirements for structures are compatible with carriages, light freight wagons and electric or diesel multiple units with an average mass per unit length over the length of each vehicle of 2.45 t/m for EN line category A, 2.75 t/m for EN line category B1, 3.1 t/m for EN line category C2 and 3.5 t/m for EN line category D2 (not for P5).

(6) The requirements for structures are compatible with 4 axle locomotive and power heads with a spacing of the axles in a bogie shall be at least 2.6 m and the average mass per unit length over the length of the vehicle of up to 5.0 t/m.

(7) Taking into account the state of art of operation there is no need to define harmonized requirements to deliver an adequate level of interoperability for these types of vehicles for the traffic code P1 and P2.

(8) For P1 and P2 lines, compliance with the High Speed Load Model (HSLM) in accordance with the specification referenced in Appendix T, Index [10] shall be stated (see procedure in point 6.2.4.10 of this UTP). If HSLM compliance cannot be shown, for the purpose of dynamic compatibility checks set out in accordance with the route compatibility check

in point 1 of Annex to UTP TCRC,	in Appendix D.1 to the TSI OPE (RINF parameter 1.1.1.1.2.4.4),

the dynamic loading, to which the compatibility with existing bridges should be checked, shall be provided in the documents with the procedure(s) as set out in

in point 1 of Annex to UTP TCRC,	RINF parameter 1.1.1.1.2.4.4

(see also procedure in point 6.2.4.10 of this UTP). When a dynamic analysis has to be undertaken with load models based on individual trains, the characteristic value of the loading for passengers or luggage carrying vehicles shall be in accordance with the design mass under normal payload in accordance with Appendix K.

(9) For avoiding excessive dynamic effects including resonance, currently it is not possible to specify harmonized minimum bridge properties to obviate the need for a dynamic appraisal. The dynamic loading from vehicles satisfying the bridge static loading requirements (specified as either a Line Category in accordance with the specification referenced in Appendix T, Index [2] or in terms of load model LM71) can in a number of cases exceed these normal bridge static loading requirements (when these static loadings are enhanced by normal industry allowances for dynamic factors for

bridge recalculation or bridge design). This risk to compatibility between vehicles and bridges is managed by the dynamic compatibility checks set out in

Point 1 of Annex to UTP TCRC	Appendix D.1 to the TSI OPE (RINF parameter 1.1.1.1.2.4.4).

When a dynamic analysis has to be undertaken with load models based on individual trains, the characteristic value of the loading for passengers or luggage carrying vehicles shall be in accordance with the design mass under normal payload in accordance with Appendix K.

(10) The requirements for loco hauled passenger trains are valid for carriages and light freight wagons satisfying EN line category A for speeds up to 200 km/h (local allowed speed) or EN line category C2 for speeds up to 160 km/h (local allowed speed).

(11) The requirements for loco hauled passenger trains are valid for carriages and light freight wagons satisfying EN line category C2 for speeds up to 160 km/h (local allowed speed).

(12) The requirements for loco hauled passenger trains are valid for carriages and light freight wagons satisfying line EN category A for speeds up to 200 km/h (local allowed speed) or EN line category B1 for speeds up to 160 km/h (local allowed speed).

(13) The requirements for loco hauled passenger trains are valid for carriages and light freight wagons satisfying EN line category B1 for speeds up to 160 km/h (local allowed speed).

(14) The requirements set out using EN line categories or load model LM71 can be fulfilled either via EN line category with the corresponding speed or with LM71 with the factor alpha in accordance with the specification referenced in Appendix T, Index [10]. The decision between the two available options, not necessarily the most onerous, is to be made exclusively by the applicant. EN line category with the corresponding speed is based on static loading multiplied by a dynamic amplification factor.

(15) Where the minimum capability requirements for a traffic code given in Table 38A are given for example in the form L > = 4 m D2 – 100 (*) and L < 4 m D2 – 200 (**), the relevant criteria in accordance with the loaded length L of the bridge element being considered shall be satisfied. EN line category with the corresponding speed is based on static loading multiplied by a dynamic amplification factor.

(*) For local allowed speeds up to 100 km/h the minimum required loading capability is D2 at the local allowed speed. For local allowed speeds exceeding 100 km/h the minimum required loading capability is D2 at 100 km/h.

(**) For local allowed speeds up to 200 km/h the minimum required loading capability is D2 at the local allowed speed.

Table 38B Loading capability requirements for geotechnical structures and earthworks (1)(2) — Passenger traffic

Traffic code	Traffic with loco hauled trains: Passenger trains including Carriages (Coaches, Vans and Car Carriers) and Light Freight Wagons and Locomotives and Power Heads (3)	Traffic with Electric or Diesel Multiple Units, Power Units and Railcars (3)
P1	n.a. (4)	D2
P2	D2	D2
P3a (> 160 km/h)	D2	C2
P3b (≤ 160 km/h)	D2	D2
P4a (> 160 km/h)	D2	C2
P4b (≤ 160 km/h)	D2	C2
P5	C2	B1
P6	a12	
P1520	Open point	
P1600	Open point	

Table 39B Loading capability requirements for geotechnical structures and earthworks — Freight traffic [2]

Traffic code	Freight trains including freight wagons, other vehicles and locomotives [2]
F1	D4
F2	D2
F3	C2
F4	B2
F1520	Open point
F1600	Open point

Notes:

(1) The published line categories of the section of line including earthworks take account of the local allowed speeds.

(2) Passenger Carriages (including Coaches, Vans, Car Carriers), Other Vehicles, Locomotives, Power Heads, Diesel and Electric Multiple Units, Power Units and Railcars are defined in point 2.2 of the UTP LOC&PAS. Light Freight Wagons are defined as vans except that they are allowed to be conveyed in formations which are not intended to convey passengers.

(3) For traffic codes P2, P3 and P4 the requirements for both traffic with loco hauled trains and traffic with multiple units shall apply. For traffic code P5, the Contracting State may indicate whether the requirements for locomotives and power heads apply.

(4) Taking into account the state of the art of operation there is no need to define harmonized requirements to deliver an adequate level of interoperability for this type of vehicles for P1 traffic codes.

APPENDIX F

Capability requirements for structures according to traffic code in the United Kingdom of Great Britain and Northern Ireland[25]

The minimum capability requirements for structures are defined in Table 40 and Table 41 according to the traffic codes given in Table 2 and Table 3. The capability requirements are defined in Table 40 and Table 41 by a combined quantity comprising of the Route Availability number and a corresponding maximum speed. The Route Availability number and associated speed shall be considered as a single combined quantity.

The Route Availability number is a function of axle load and geometrical aspects relating to the spacing of axles. Route Availability numbers are defined in the national technical requirements notified for this purpose.

Table 40 Route Availability number –Associated Speed (1) (5) (miles per hour) — Passenger traffic

Traffic code	Passenger Carriages (including Coaches, Vans and Car Carriers) and Light Freight Wagons (2) (3) (6)	Locomotives and Power Heads (2) (4)	Electric or Diesel Multiple Units, Power Units and Railcars (2) (3) (6)
P1	n.a. (11)	n.a. (11)	Open point
P2	n.a. (11)	n.a. (11)	Open point
P3a (> 160 km/h)	RA1 – 125 RA2 – 90	RA7 – 125 (7) RA8 – 110 (7) RA8 – 100 (8) RA5 – 125 (9)	Open point
P3b (≤ 160 km/h)	RA1 – 100 RA2 – 90	RA8 – 100 (8) RA5 – 100 (9)	RA3 – 100
P4a (> 160 km/h)	RA1 – 125 RA2 – 90	RA7 – 125 (7) RA7 – 100 (8) RA4 – 125 (9)	Open point
P4b (≤ 160 km/h)	RA1 – 100 RA2 – 90	RA7 – 100 (8) RA4 – 100 (9)	RA3 – 100
P5	RA1 – 75	RA5 – 75 (8) (10) RA4 – 75 (9) (10)	RA3 – 75
P6	RA1		
P1600	Open point		

25 Title of the INF TSI reads: “Capability requirements for structures according to traffic code in the United Kingdom (Northern Ireland)”

Table 41 Route Availability number –Associated Speed [(1) (5)] (miles per hour) — Freight traffic

Traffic code	Freight wagons and other vehicles	Locomotives [(2) (4) (8)]
F1	RA8 – 75	RA7 – 75
F2	RA7 – 75	RA7 – 75
F3	RA5 – 60	RA7 – 60
F4	RA4 – 60	RA5 – 60
F1600	Open point	

Notes:

(1) The indicated speed value in the table represents the maximum requirement for the line and may be lower in accordance with the requirements in point 4.2.1 (12). When checking individual structures on the line, it is acceptable to take account of the type of vehicle and local allowed speed.

(2) Passenger Carriages (including Coaches, Vans, Car Carriers), Other Vehicles, Locomotives, Power Heads, Diesel and Electric Multiple Units, Power Units and Railcars are defined in the UTP LOC & PAS. Light Freight Wagons are defined as vans except that they are allowed to be conveyed in formations which are not intended to convey passengers.

(3) The requirements for structures are compatible with Passenger Coaches, Vans, Car Carriers, Light Freight Wagons and vehicles in Diesel and Electric Multiple Units and Power Units with a length of; 18 m to 27,5 m for conventional and articulated vehicles and with a length of 9 m to 14 m for regular single axles.

(4) The requirements for structures are compatible with up to two adjacent coupled locomotives and/or power heads. The requirements for structures are compatible up to a maximum speed of 75 mph for up to five adjacent coupled locomotives and/or power heads (or a train of locomotives and/or power heads) subject to the locomotives and/or power heads satisfying the corresponding limits for freight wagons.

(5) When checking the compatibility of individual trains and structures, the basis of the compatibility check shall be in accordance with Appendix K except where modified by the national technical requirements notified for this purpose.

(6) The requirements for structures are compatible with an average mass per unit length over the length of each coach/vehicle of 3,0 t/m

(7) Only 4 axle vehicles allowed. The spacing of the axles in a bogie shall be at least 2,6 m. The average mass per unit length over the length of the vehicle shall not exceed 4,6 t/m.

(8) 4 or 6 axle vehicles allowed.

(9) Powerhead, only 4 axle vehicles allowed. Also includes locomotives where difference in length between locomotive and hauled vehicles is less than 15 % of length of hauled vehicles for speeds over 90 mph.

(10) For traffic code P5 the Contracting State may indicate whether the requirements for locomotives and power heads apply.

(11) Taking into account the state of art of operation there is no need to define harmonized requirements to deliver an adequate level of interoperability for this type of vehicles for P1 and P2 traffic codes.

APPENDIX G

Speed conversion to miles per hour for Ireland and the United Kingdom of Great Britain and Northern Ireland[26]

Table 42 Speed conversion from (km/h) to (mph)

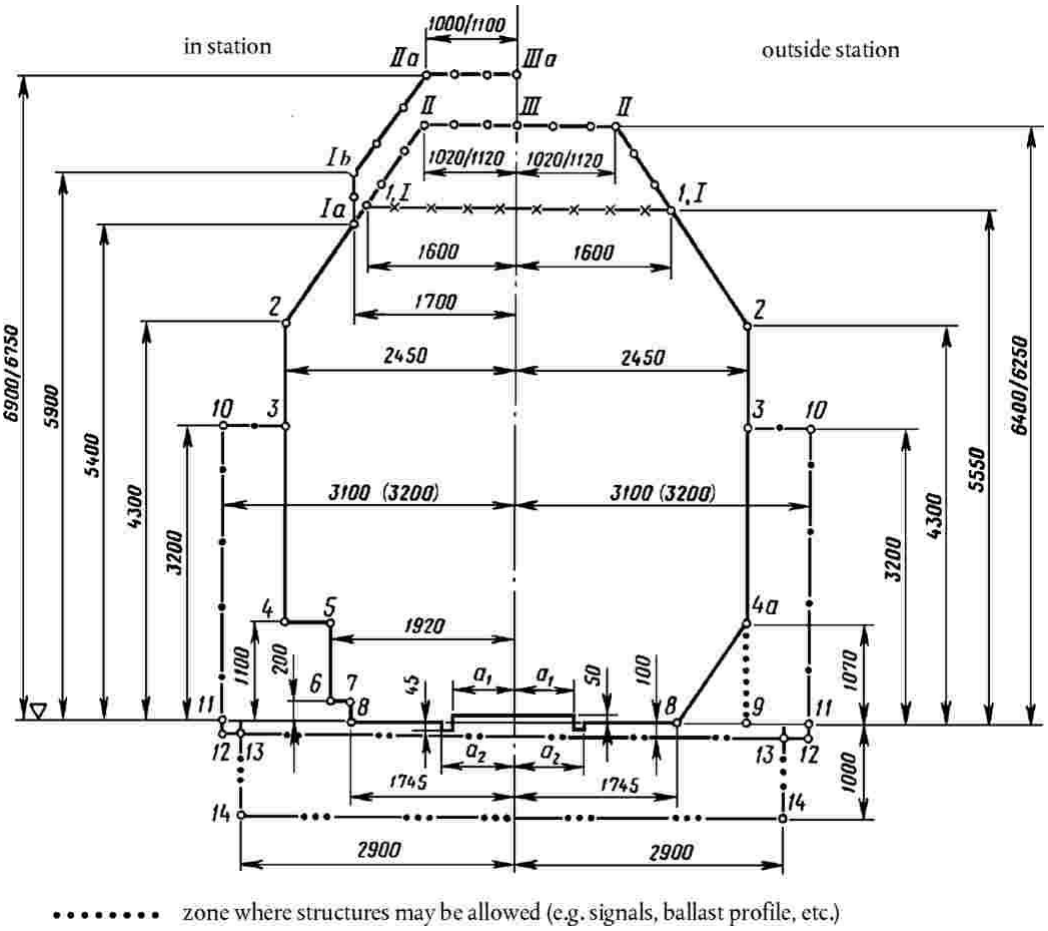
Speed [km/h]	Speed [mph]
2	1
3	1
5	3
10	5
15	10
20	10
30	20
40	25
50	30
60	40
80	50
100	60
120	75
140	90
150	95
160	100
170	105
180	110
190	120
200	125
220	135
225	140
230	145
250	155
280	175
300	190
320	200
350	220

[26] The title in the INF TSI reads: ”Speed conversion to miles per hour for Ireland and United Kingdom (Northern Ireland)”

APPENDIX H

Structure gauge for the 1 520 mm track gauge system

Figure 3 Structure gauge S for the 1 520 mm track gauge system (dimensions in mm)



Clarifications for Figure 3:

All horizontal dimensions shall be measured from the centre of the track, and all vertical dimensions shall be measured from the top of the rail head level.

Left side of contour — applications for tracks in the railway station, stop/halt and for branch tracks/industry track (except contour Ia, Ib, IIa, IIIa),

Right side of contour — applications for tracks on the plain line.

Application of specific parts of the contour:

1,I — 1, I — contour of structure gauge for non-electrified tracks,

1,I — II — III — II — 1,I — contour of structure gauge for electrified tracks — for tracks on the plain (open) line and for tracks in the railway station and for branch/industry tracks, where standing of vehicles is not expected,

Ia — Ib — IIa — IIIa — contour of structure gauge for electrified tracks — for other station tracks and other branch/industry tracks

Note: Values of 1 000 mm, 1 020 mm, 6 900 mm and 6 400 mm given in the numerators are for contact system with carrying cable.

Values of 1 100 mm, 1 120 mm, 6 750 mm and 6 250 mm given in the denominator are for contact system without carrying cable,

11 — 10 — 3 — contour of structure gauge for structures and equipment (except tunnel, bridge, platform, ramp) on the outside of 'edge' tracks;

9 — 4a — contour of structure gauge for tunnel, for railing on the bridge, elevated track (ballast profile), signals, embankment wall and for railing on the other structures of railway subgrade,

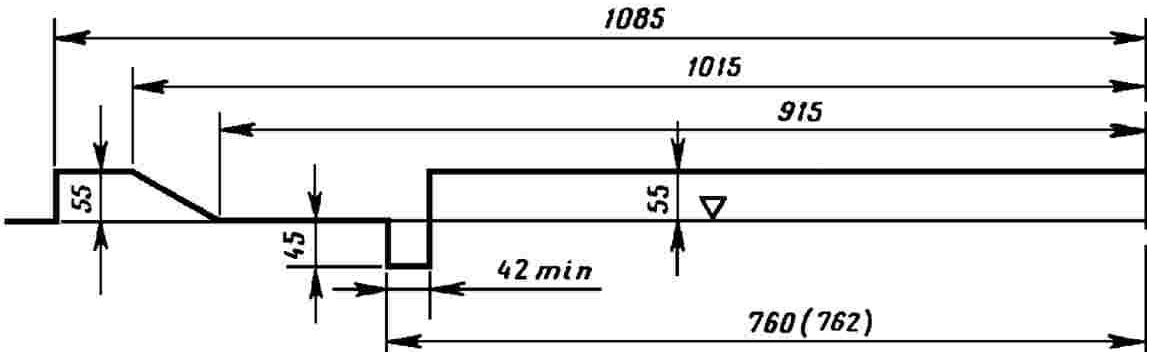
12-12 — contour from which (on track between stations or in stations within usable length of track) any device could not be above (higher), except level crossing covering, locomotive signalling inductors, switches mechanism and their near situated signalling and safety equipment

14-14 — contour of building (or foundation), underground cables, steel cables, pipes and other not railway structures (except signalling and safety equipment)

For nominal track gauge of 1 520 mm a_1 = 670 mm and a_2= 760 mm.

For nominal track gauge of 1 524 mm a_1 = 672 mm and a_2= 762 mm.

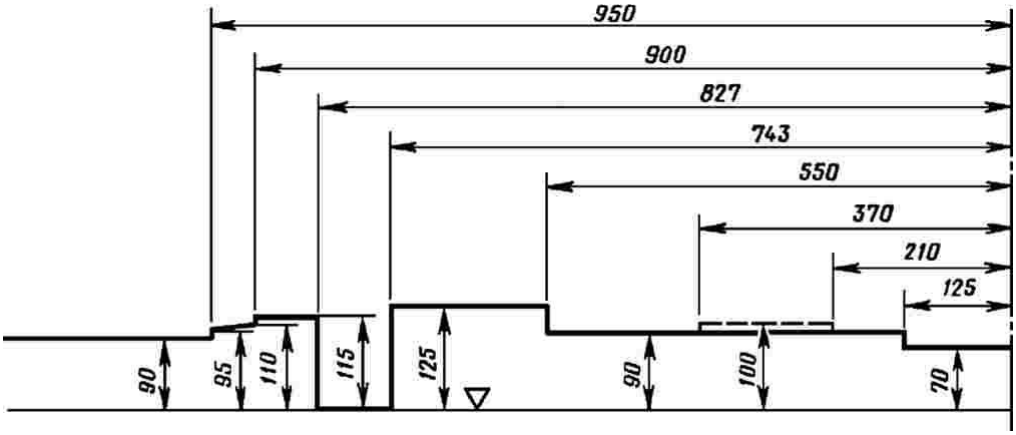
Figure 4 Reference profile of the lower parts on tracks fitted with double slip



Clarification for Figure 4:

The distance of 760 mm is for track gauge 1 520 mm, and 762 mm for track gauge 1 524 mm.

Figure 5 Reference profile of the lower parts on marshalling yards fitted with rail brakes



APPENDIX I

Not used

APPENDIX J

Safety assurance over fixed obtuse crossings

(J.1) The fixed obtuse crossings should be designed in order not to have a too long unguided length. In obtuse crossing check rails cannot be constructed to assure guidance over the whole length. This unguided length can be accepted up to a certain limit, defined by a reference situation defining:

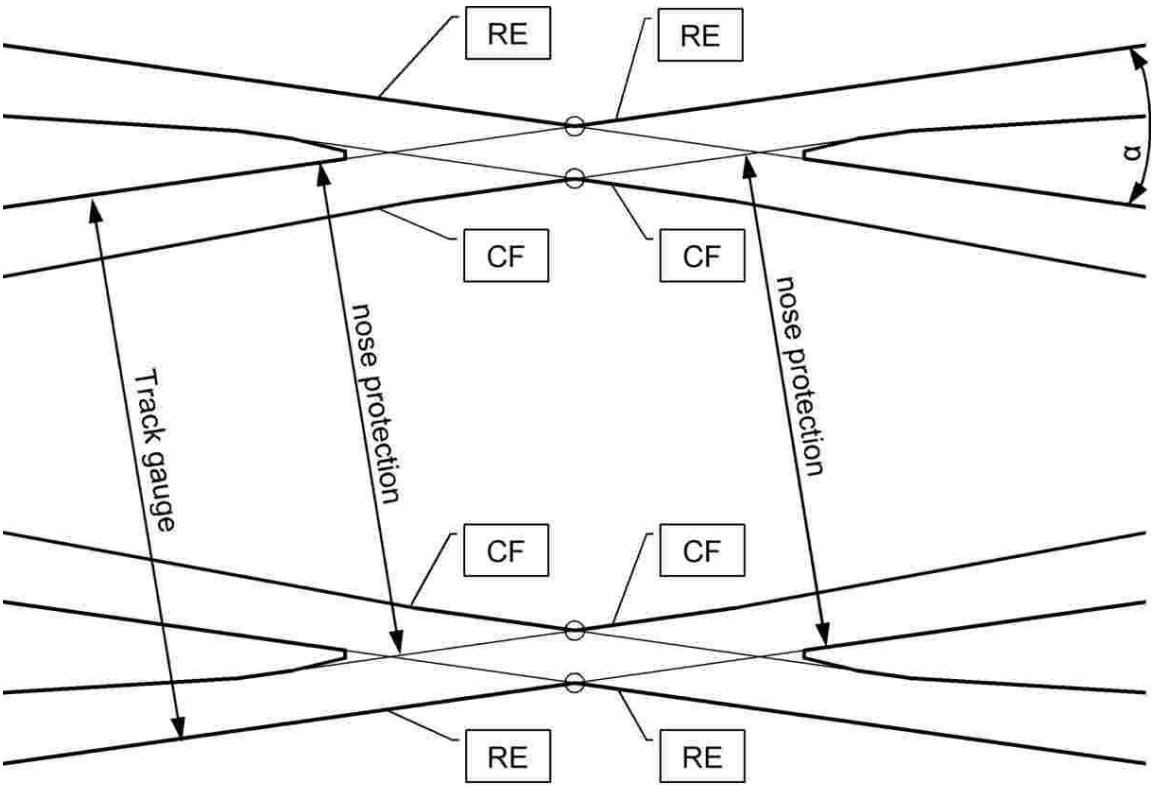
a) Minimum crossing angle: tangent 1 in 9 (tgα = 0,11, α = 6°20′)

b) Minimum radius through obtuse crossing: 450 m

c) Minimum height of check rail: 45 mm

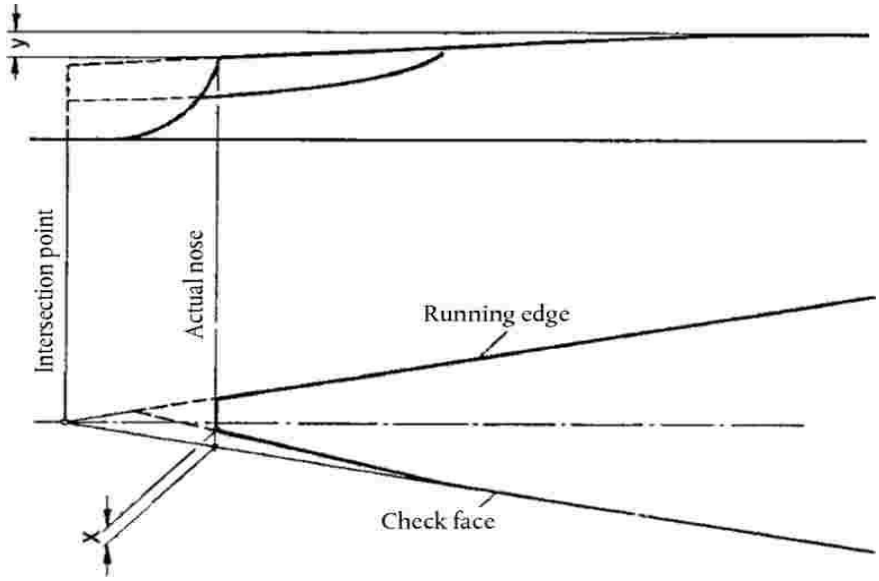
d) Nose shape as defined in the figure below

Figure 6 Obtuse crossing



RE = running edge
CF = check face (guiding edge)

Figure 7 Point retraction X on check face



X = 3 mm (over a length of 150 mm).

Y = 8 mm (over a length of 200 to 500 mm approximately)

(J.2) If one or more of the above requirements is not respected, the design shall be checked, verifying either the equivalence of the unguided length or acceptance of the interference between wheel and nose when they get in contact.

(J.3) The design shall be checked for wheels with diameter between 630 mm and 840 mm. For wheel diameters between 330 mm and 630 mm specific demonstrations are required.

(J.4) The following graphs allow simple verification of unguided length for specific situation with different crossing angles, height of check rail and different crossing curvature.

The graphs consider the following maximum track tolerances:

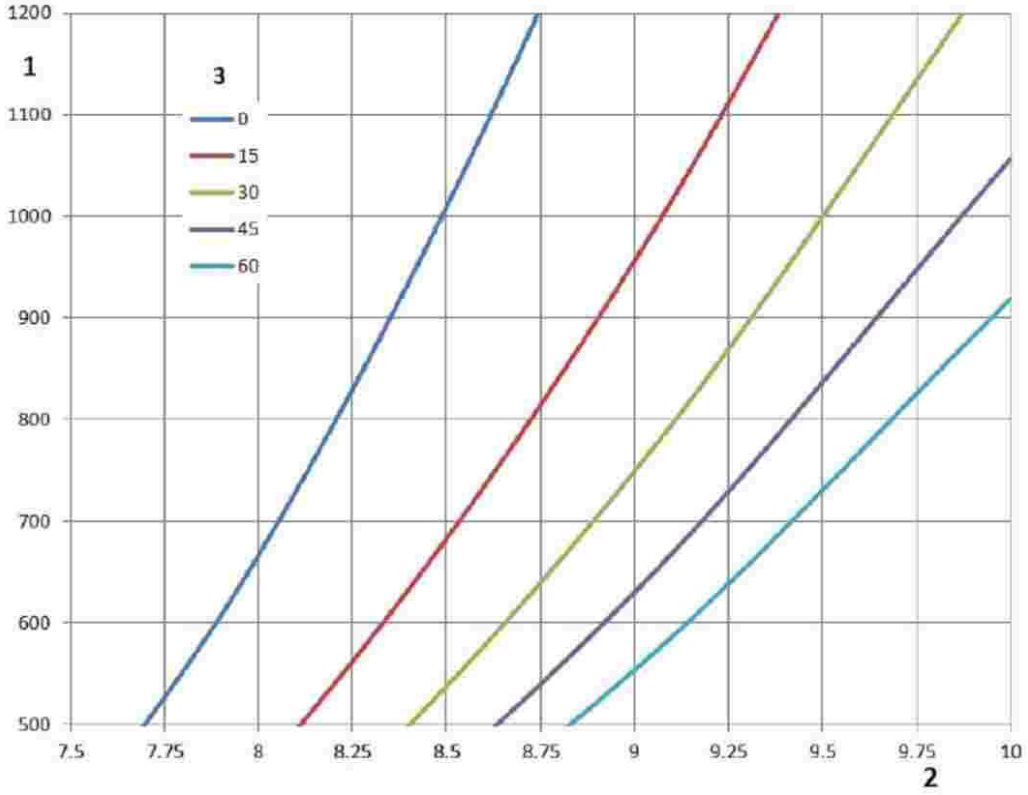
- Track gauge between 1 433 mm and 1 439 mm inclusive
- Nose protection between 1 393 mm and 1 398 mm inclusive
- Free wheel passage ≤ 1 356 mm

Figure 8 allows to specify the minimum wheel diameter that can run on curved obtuse crossings with a radius of 450 m, Figure 9 allows it for straight obtuse crossings.

For other situations specific calculations can be performed.

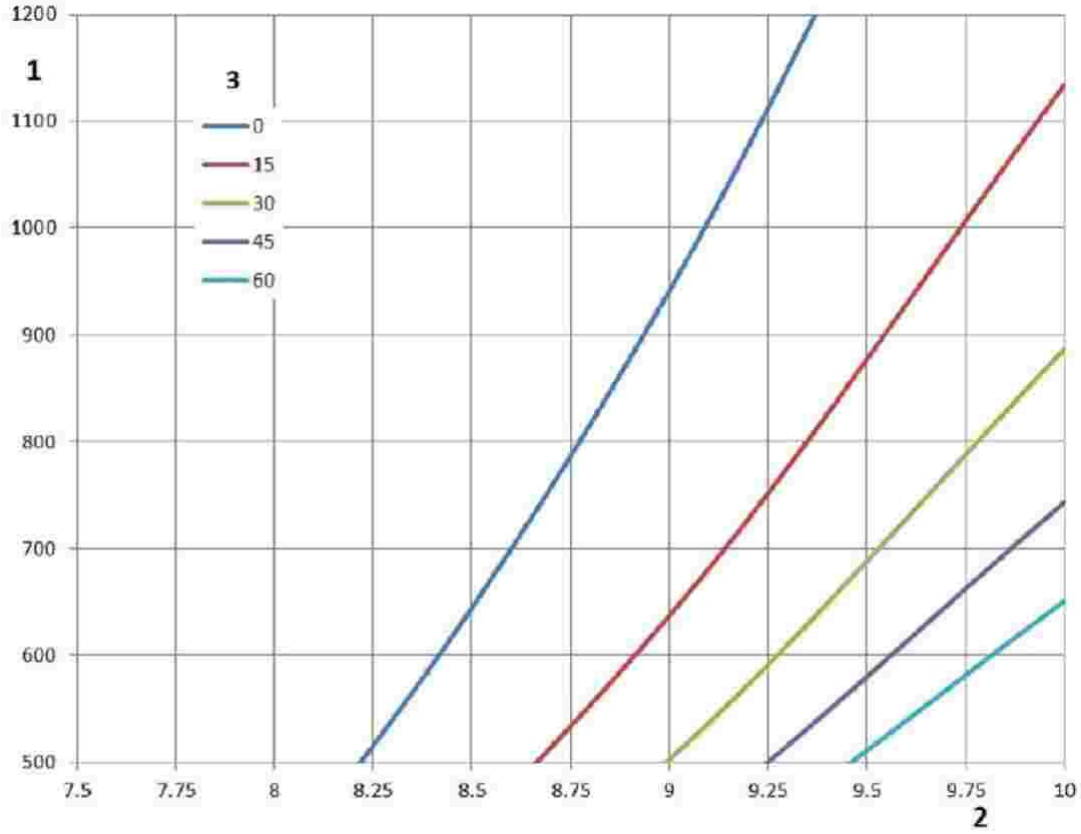
(J.5) For track gauge systems other than 1 435 mm, specific calculations shall be performed.

Figure 8 Minimum wheel diameter against crossing angle for 450 m radius of obtuse crossing



1. Minimum wheel diameter (mm)
2. N for crossing angle tangent 1 in N
3. Height of check rail (mm) (Z3)

Figure 9 Minimum wheel diameter against crossing angle for straight obtuse crossing



1. Minimum wheel diameter (mm)
2. N for crossing angle tangent 1 in N
3. Height of check rail (mm) (Z3)

APPENDIX K

Basis of minimum requirements for structures for passenger carriages and multiple units

The following mass definitions for passenger carriages and multiple units form the basis of the minimum dynamic requirements for structures and checking the compatibility of structures with passenger carriages and multiple units.

Where a dynamic appraisal is required to determine the load carrying capacity of the bridge, the load carrying capacity of the bridge shall be specified and expressed in terms of the design mass under normal payload in accordance with the specification referenced in Appendix T, Index [1], taking into account the values for passenger payload in standing areas given in Table 45.

Mass definitions for static compatibility are based upon the design mass under exceptional payload established in accordance with the specification referenced in Appendix T, Index [1], taking into account the specification referenced in Appendix T, Index [2].

Table 45 Passenger payload in standing areas in kg/m^2
in accordance with the specification referenced in Appendix T, Index [1]

Type of trains	**Normal payload** to specify Dynamic Compatibility
High speed and long distance trains	160 (1)
High speed and long distance trains Reservation Obligatory	0
Others (regional, commuter, suburban trains)	280

Notes:

(1) Normal payload of the specification referenced in Appendix T, Index [1], plus an additional 160 kg/m^2 for standing areas

APPENDIX L

Not used

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APPENDIX M

Not used

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APPENDIX N

Not used

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APPENDIX O

Specific case on the Ireland and United Kingdom of Northern Ireland networks

Rules and drawings related to gauges IRL1, IRL2 and IRL3 are an open point.

APPENDIX P

Not used

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APPENDIX Q[27]

National technical requirements for UK-GB Specific Cases

The national technical requirements for UK-GB specific cases referred to in point 7.7.1 of this UTP are contained in the documents listed in Table 47. All documents are available on www.rgsonline.co.uk.

Table 47 Notified national technical requirements for UK-GB Specific Cases

Specific Case	UTP Point	Requirement	NTR Ref	NTR Title
7.7.1.1	4.2.1: Table 2 & Table 3	Categories of line: Gauge	GI/RT7073	Requirements for the Position of Infrastructure and for Defining and Maintaining Clearances
			GE/RT8073	Requirements for the Application of Standard Vehicle Gauges
			GI/RT7020	GB Requirements for Platform Height, Platform Offset and Platform Width
7.7.1.2 & 7.7.1.10	4.2.3.1 & 6.2.4.1	Structure gauge	GI/RT7073	Requirements for the Position of Infrastructure and for Defining and Maintaining Clearances
			GE/RT8073	Requirements for the Application of Standard Vehicle Gauges
			GI/RT7020	GB Requirements for Platform Height, Platform Offset and Platform Width
7.7.1.3 & 7.7.1.11	4.2.3.2: Table 4 & 6.2.4.2	Distance between track centres	GI/RT7073	Requirements for the Position of Infrastructure and for Defining and Maintaining Clearances
7.7.1.5	4.2.5.3 & Appendix J	Maximum unguided length of fixed obtuse crossings	GC/RT5021	Track System Requirements
			GM/RT2466	Railway Wheelsets
7.7.1.7	4.2.9.2	Platform height	GI/RT7020	GB Requirements for Platform Height, Platform Offset and Platform Width
7.7.1.8 & 7.7.1.12	4.2.9.3 & 6.2.4.11	Platform offset	GI/RT7020	GB Requirements for Platform Height, Platform Offset and Platform Width
			GI/RT7073	Requirements for the Position of Infrastructure and for Defining and Maintaining Clearances

[27] Appendix Q of the INF TSI reads: (Not used)

APPENDIX R

List of open points

(1) Immediate action limits for isolated defects in alignment for speeds of more than 300 km/h (4.2.8.1).

(2) Immediate action limits for isolated defects in longitudinal level for speeds of more than 300 km/h (4.2.8.2).

(3) The minimum allowed value of distance between track centres for the uniform structure gauge IRL3 is an open point (7.7.18.2).

(4) EN Line Category –Associated Speed [km/h] for Traffic codes P1520 (all vehicles), P1600 (all vehicles), F1520 (all vehicles) and F1600 (all vehicles) in Appendix E, Tables 38A, 39A, 38B and 39B.

(5) Route Availability Number – Associated Speed [miles/h] for Traffic codes P1 (multiple units), P2 (multiple units), P3a (multiple units), P4a (multiple units), P1600 (all vehicles) and F1600 (all vehicles) in Appendix F, Tables 40 and 41.

(6) Rules and drawings related to gauges IRL1, IRL2 and IRL3 are an open point (Appendix O).

(7) The requirements for mitigating the risk for ballast pick up for speed greater than 250 km/h.

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APPENDIX S

Glossary

Table 48 Terms

Defined term	UTP point	Definition
Actual point (RP)/ Praktischer Herzpunkt/ Pointe de cœur réelle	4.2.8.6	Physical end of a crossing vee. See Figure 2, which shows the relationship between the actual point (RP) and the intersection point (IP).
Alert limit/ Auslösewert/ Limite d'alerte	4.5.2	Refers to the value which, if exceeded, requires that the track geometry condition is analysed and considered in the regularly planned maintenance operations.
Axle load/ Achsfahrmasse/ Charge à l'essieu	4.2.1, 4.2.6.1	Sum of the static vertical wheel forces exerted on the track through a wheelset or a pair of independent wheels divided by acceleration of gravity.
Braking systems independent of wheel-rail adhesion conditions/ Von den Bedingungen des RadSchiene-Kraftschlusses unabhängige Bremssysteme/ Systèmes de freinage indépendant des conditions d'adhérence roue-rail	4.2.6.2.2	"Braking systems independent of wheel – rail adhesion conditions" refers to all brake systems of the rolling stock capable to develop a brake force applied to the rails independently of the wheel – rail adhesion conditions (e.g. magnetic braking systems and eddy current braking systems)
Cant/ Überhöhung/ Dévers de la voie	4.2.4.2 4.2.8.5	Difference in height, relative to the horizontal, of the two rails of one track at a particular location, measured at the centrelines of the heads of the rails.
Cant deficiency/ Überhöhungsfehlbetrag/ Insuffisance de devers	4.2.4.3	Difference between the applied cant and a higher equilibrium cant.
Common crossing/ Starres Herzstück/ Cœur de croisement	4.2.8.6	Arrangement ensuring intersection of two opposite running edges of turnouts or diamond crossings and having one crossing vee and two wing rails.
Crosswind/ Seitenwind/ Vents traversiers	4.2.10.2	Strong wind blowing laterally to a line which may adversely affect the safety of trains running.
Design value/ Planungswert/ Valeur de conception	4.2.3.4, 4.2.4.2, 4.2.4.5, 4.2.5.1, 4.2.5.3	Theoretical value without manufacturing, construction or maintenance tolerances.
Design track gauge/ Konstruktionsspurweite/ Écartement de conception de la voie	5.3.3	A single value which is obtained when all the components of the track conform precisely to their design dimensions or their median design dimension when there is a range.

Defined term	UTP point	Definition
Distance between track centres/ Gleisabstand/ Entraxe de voies	4.2.3.2	The distance between points of the centre lines of the two tracks under consideration, measured parallel to the running surface of the reference track namely the less canted track.
Dynamic lateral force/ Dynamische Querkraft/ Effort dynamique transversal	4.2.6.3	The sum of dynamic forces exerted by a wheelset on the track in lateral direction.
Earthworks/ Erdbauwerke/ Ouvrages en terre	4.2.7.2, 4.2.7.4	Soil structures and soil-retaining structures that are subject to railway traffic loading.
EN Line Category/ EN Streckenklasse/ EN Catégorie de ligne	4.2.7.4, Appendix E	The result of the classification process set out in the specification referenced in Appendix T, Index [2] and referred to in that standard as “Line Category”. It represents the ability of the infrastructure to withstand the vertical loads imposed by vehicles on the line or section of line for regular (“normal”) service.
Equivalent conicity/ Äquivalente Konizität/ Conicité équivalente	4.2.4.5, 4.2.11.2	The tangent of the cone angle of a wheelset with coned wheels whose lateral movement has the same kinematic wavelength as the given wheelset on straight track and large-radius curves.
Fixed nose protection/ Leitweite/ Cote de protection de pointe	4.2.5.3, Appendix J	Dimension between the crossing nose and check rail (see dimension No 2 on Figure 14).
Flangeway depth/ Rillentiefe/ Profondeur d’ornière	4.2.8.6.	Dimension between the running surface and the bottom of flangeway (see dimension No 6 on Figure 14).
Flangeway width/ Rillenweite/ Largeur d’ornière	4.2.8.6.	Dimension between a running rail and an adjacent check or wing rail (see dimension No 5 on Figure 14).
Free wheel passage at check rail/wing rail entry/ Freier Raddurchlauf im Radlenker-Einlauf/Flügelschienen-Einlauf/ Cote d’équilibrage du contre-rail	4.2.8.6.	Dimension between the working face of the crossing check rail or wing rail and the gauge face of the running rail opposite across the gauge measured at entry to check rail or wing rail respectively. (see dimensions No 4 on Figure 14). The entry to the check rail or wing rail is the point at which the wheel is allowed to contact the check rail or wing rail.
Free wheel passage at crossing nose/ Freier Raddurchlauf im Bereich der Herzspitze/ Cote de libre passage dans le croisement	4.2.8.6.	Dimension between the working face of the crossing wing rail and check rail opposite across the gauge (see dimension No 3 on Figure 14).

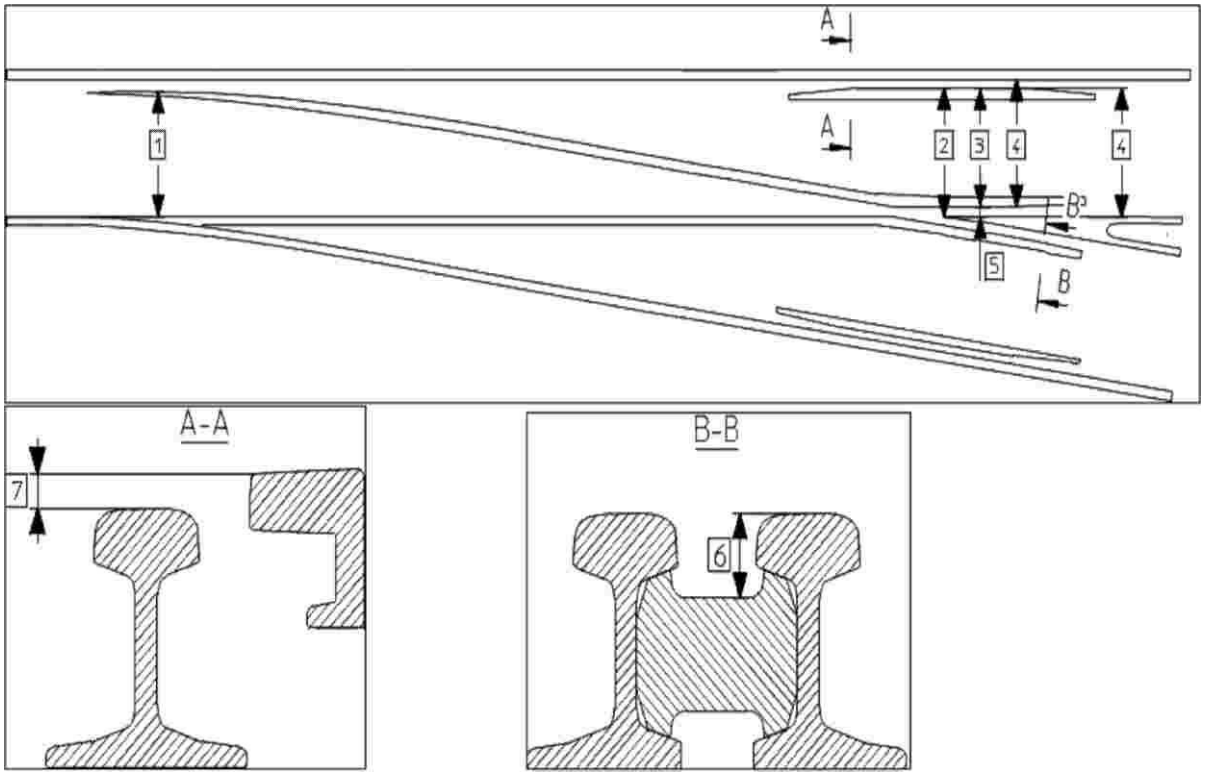
Defined term	UTP point	Definition
Free wheel passage in switches/ Freier Raddurchlauf im Bereich der Zungen-vorrichtung/ Cote de libre passage de l'aiguillage	4.2.8.6.	Dimension from the gauge face of one switch rail to the back edge of the opposite switch rail (see dimension No 1 on Figure 14).
Gauge/ Begrenzungslinie/ Gabarit	4.2.1, 4.2.3.1	Set of rules including a reference contour and its associated calculation rules allowing definition of the outer dimensions of the vehicle and the space to be cleared by the infrastructure.
Geotechnical structures/ Geotechnische Strukturen/ Structures géotechniques	4.2.7.2, 4.2.7.4	A structure that includes ground or structural member that relies on the ground resistance. Note: Earthworks is a subset for geotechnical structure
HBW/HBW/HBW	5.3.1.2	The non SI unit for steel hardness defined in the specification referenced in Appendix T, Index [16].
Height of check rail/ Radlenkerüberhöhung/ Surélévation du contre rail	4.2.8.6, Appendix J	Height of the check rail above the running surface (see dimension 7 on Figure 14).
Immediate Action Limit/ Soforteingriffsschwelle/ Limite d'intervention immédiate	4.2.8, 4.5	The value which, if exceeded, requires taking measures to reduce the risk of derailment to an acceptable level.
Infrastructure Manager/ Betreiber der Infrastruktur/ Gestionnaire de l'infrastructure	4.2.5.1, 4.2.8.3, 4.2.8.6, 4.2.11.2, 4.4, 4.5.2, 4.6, 4.7, 6.2.2.1, 6.2.4, 6.4	As defined in Article 2 letter k) of ATMF. [For EU: As defined in Article 3(2) of 2012/34/EU of the European Parliament and of the Council of 21 November 2012 establishing a single European railway area
In service value/ Wert im Betriebszustand/ Valeur en exploitation	4.2.8.5, 4.2.11.2	Value measured at any time after the infrastructure has been placed into service.
Intersection point (IP)/ Theoretischer Herzpunkt/ Point d'intersection théorique	4.2.8.6	Theoretical intersection point of the running edges at the centre of the crossing (see figure 2).
Intervention Limit/ Eingriffsschwelle/ Valeur d'intervention	4.5.2	The value, which, if exceeded, requires corrective maintenance in order that the immediate action limit shall not be reached before the next inspection;
Isolated defect/ Einzelfehler/ Défaut isolé	4.2.8	A discrete track geometry fault.
Line speed/ Streckengeschwindigkeit/ Vitesse de la ligne	4.2.1	Maximum speed for which a line has been designed.

Defined term	UTP point	Definition
Maintenance file/ Instandhaltungsdossier/ Dossier de maintenance	4.5.1	Elements of the technical file relating to conditions and limits of use and instructions for maintenance.
Maintenance plan/ Instandhaltungsplan/ Plan de maintenance	4.5.2	A series of documents setting out the infrastructure maintenance procedures adopted by an Infrastructure Manager.
Multi-rail track/ Mehrschienengleis/ Voie à multi écartement	4.2.2.2	Track with more than two rails, where at least two pairs of respective rails are designed to be operated as separate single tracks, with or without different track gauges.
Nominal track gauge/ Nennspurweite/ Écartement nominal de la voie	4.2.4.1	A single value which identifies the track gauge but may differ from the design track gauge.
Normal service/ Regelbetrieb/ Service régulier	4.2.2.2 4.2.9	The railway operating to a planned timetable service.
Passive provision/ Vorsorge für künftige Erweiterungen/ Réservation pour extension future	4.2.9	Provision for the future construction of a physical extension to a structure (for example: increased platform length).
Performance Parameter/ Leistungskennwert/ Paramètre de performance	4.2.1	Parameter describing a UTP Category of Line used as the basis for the design of infrastructure subsystem elements and as the indication of the performance level of a line.
Plain line/ Freie Strecke/ Voie courante	4.2.4.5 4.2.4.6 4.2.4.7	Section of track without switches and crossings.
Point retraction/ Spitzenbeihobelung/ Dénivelation de la pointe de cœur	4.2.8.6	The reference line in a fixed common crossing can deviate from the theoretical reference line. From a certain distance to the crossing point, the reference line of the vee can, depending on the design, be retracted from this theoretical line away from the wheel flange in order to avoid contact between both elements. This situation is described in Figure 2.
Rail inclination/ Schienenneigung/ Inclinaison du rail	4.2.4.5 4.2.4.7	An angle defining the inclination of the head of a rail when installed in the track relative to the plane of the rails (running surface), equal to the angle between the axis of symmetry of the rail (or of an equivalent symmetrical rail having the same rail head profile) and the perpendicular to the plane of the rails.
Rail pad/ Schienenzwischenlage/ Semelle sous rail	5.3.2	A resilient layer fitted between a rail and the supporting sleeper or baseplate.

Defined term	UTP point	Definition
Reverse curve/ Gegenbogen/ Courbes et contre-courbes	4.2.3.4	Two abutting curves of opposite flexure or hand
Structure gauge/ Lichtraum/ Gabarit des obstacles	4.2.3.1	Defines the space in relation to the reference track that shall be cleared of all objects or structures and of the traffic on the adjacent tracks, in order to allow safe operation on the reference track. It is defined on the basis of the reference contour by application of the associated rules.
Swing nose/ Bewegliche Herzstückspitze/ Cœur à pointe mobile	4.2.5.2	Within the domain of "common crossing with movable point", the term "swing nose" identifies the part of the crossing which forms the vee and that it is moved to form a continuous running edge for either the main or the branch line.
Switch/ Zungenvorrichtung/ Aiguillage	4.2.8.6	A unit of track comprising two fixed rails (stock rails) and two movable rails (switch rails) used to direct vehicles from one track to another track.
Switches and crossings/ Weichen und Kreuzungen/ Appareils de voie	4.2.4.5, 4.2.4.7, 4.2.5, 4.2.6, 4.2.8.6, 5.2, 6.2.4.4, 6.2.4.8, 6.2.5.2, 7.3.3, Appendix C and D,	Track constructed from sets of switches and individual crossings and the rails connecting them.
Through route/ Stammgleis/ Voie directe	Appendix D	In the context of switches and crossings a route which perpetuate the general alignment of the track.
Track design/ Oberbaukonstruktion/ Conception des voies	4.2.6, 6.2.5, Appendix C and D	The track design consists of cross-section defining basic dimensions and track components (for example rail, rail fastenings, sleepers, ballast) used together with operating conditions with an impact on forces related to 4.2.6, such as axle load, speed and radius of horizontal curvature.
Track gauge/ Spurweite/ Écartement de la voie	4.2.4.1, 4.2.4.5, 4.2.8.4, 5.3.3, 6.1.5.2, 6.2.4.3, Appendix H	The smallest distance between lines perpendicular to the running surface intersecting each rail head profile in a range from 0 to 14 mm below the running surface.
Track twist/ Gleisverwindung/ Gauche	4.2.7.1.6, 4.2.8.3, 6.2.4.9,	Track twist is defined as the algebraic difference between two cross levels taken at a defined distance apart, usually expressed as a gradient between the two points at which the cross level is measured.
Train length/ Zuglänge/ Longueur du train	4.2.1	The length of a train, which can run on a certain line in normal operation.
Unguided length of an obtuse crossing/ Führungslose Stelle/ Lacune dans la traversée	4.2.5.3, Appendix J	Portion of obtuse crossing where there is no guidance of the wheel described as "unguided distance" in the specification referenced in Appendix T, Index [17].

Defined term	UTP point	Definition
Usable length of a platform/ Bahnsteignutzlänge/ Longueur utile de quai	4.2.1, 4.2.9.1	The maximum continuous length of that part of platform in front of which a train is intended to remain stationary in normal operating conditions for passengers to board and alight from the train, making appropriate allowance for stopping tolerances. Normal operating conditions means that railway is operating in a non-degraded mode (e.g. rail adhesion is normal, signals are working, everything is working as planned).

Figure 14 Geometry of switches and crossings



(1) 1 Free wheel passage in switches
(2) Fixed nose protection
(3) Free wheel passage at crossing nose
(4) Free wheel passage at check rail/wing rail entry
(5) Flangeway width
(6) Flangeway depth
(7) Height of check rail

APPENDIX T

Technical specifications referenced in this UTP

Table 49 Referenced standards

Index	Characteristics to be assessed	UTP Point	Mandatory standard point
[1]	**EN 15663:2017+A1:2018** **Railway applications – Vehicle reference masses**		
[1.1]	Mass definition of rolling stock	4.2.1(7), Table 2 Appendix K	4.5
[1.2]	Mass definition of rolling stock	4.2.1(7), Table 3	4.5 and 7.4
[1.3]	Passenger payload for high speed and long distance trains	Appendix K, Table 45	Table 7
[1.4]	Passenger payload for other trains	Appendix K, Table 45	Table 8
[2]	**EN 15528:2021** **Railway applications – Line categories for managing the interface between load limits of vehicles and infrastructure**		
[2.1]	Mass definition of rolling stock	4.2.1(7), Table 2 Appendix K	6.4
[2.2]	Capability requirements for existing structures in accordance with traffic code	Appendix E	Annex A
[2.3]	Line categories	Appendix E, Table 38A (note [9])	
[2.4]	Definition of line category	Appendix S	5
[3]	**EN 15273-3:2013+A1:2016** **Railway applications – Gauges – Part 3: structure gauges**		
[3.1]	Structure gauge	4.2.3.1(1)	Annex C and in Annex D, point D.4.8
[3.2]	Structure gauge	4.2.3.1(2)	Annex C
[3.3]	Structure gauge – Assessment	4.2.3.1(3), 6.2.4.1	5, 7, 10 Annex C and in Annex D, point D.4.8
[3.4]	Distance between track centres – Assessment	4.2.3.2(3), 6.2.4.2	9
[3.5]	Platform offset – Assessment	4.2.9.3(1), 6.2.4.11(1)	13
[3.6]	Calculation of the structure gauge for the lower parts for the 1 668 mm track gauge	Appendix P	5, 7 and 10
[4]	**EN 13803:2017** **Railway applications – Track – Track alignment design parameters – Track gauges 1 435 mm and wider**		
[4.1]	Minimum radius of horizontal curve Definition of reference vehicle	4.2.3.4(2)	Tables N.1 and N.2

Index	Characteristics to be assessed	UTP Point	Mandatory standard point
[4.2]	Upgrading or renewal of the infrastructure, for parameters cant and cant deficiency	7.3.2	6.2 (Table 5) and 6.3 (table 7 for non-tilting trains) (see also corresponding notes in both chapters).
[5]	**EN 15302:2021** **Railway applications – Wheel-rail contact geometry parameters – Definitions and methods for evaluation**		
[5.1]	Equivalent conicity	4.2.4.5(4)	6, 8, 9, 12
[5.2]	Assessment	6.2.4.6	6, 8, 9, 12
[6]	**EN 13715:2020** **Railway applications – Wheelsets and bogies – Wheels – Tread profile**		
[6.1]	Equivalent conicity	4.2.4.5(4)(a) and (b)	Annex C
[6.2]	Equivalent conicity	4.2.4.5(4)(c) and (d)	Annex B
[7]	**EN 13674-1:2011+A1:2017** **Railway applications – Track – Rail – Part 1: Vignole railway rails 46 kg/m and above**		
[7.1]	Railhead profile for plain line	4.2.4.6(1)	Annex A
[7.2]	Assessment of rails	6.1.5.1(a)	9.1.8
[7.3]	Assessment of rails	6.1.5.1(b)	9.1.9
[7.4]	Assessment of rails	6.1.5.1(c)	8.1 and 8.4
[8]	**EN 13674-4:2006+A1:2009** **Railway applications – Track – Rail – Part 4: Vignole railway rails from 27 kg/m to, but excluding 46 kg/m**		
[8.1]	Railhead profile for plain line	4.2.4.6(1)	Annex A
[9]	**EN 14363:2016+A2:2022** **Railway applications – Testing and Simulation for the acceptance of running characteristics of railway vehicles – Running Behaviour and stationary tests**		
[9.1]	Track resistance to vertical loads Lateral track resistance	4.2.6.1(b) and (c) 4.2.6.3(b)	7.5.3
[9.2]	Lateral track resistance	4.2.6.3(a)	7.5.2 and Table 4
[10]	**EN 1991-2:2003/AC:2010** **Eurocode 1: Actions on structures – Part 2: Traffic loads on bridges**		
[10.1]	Structures resistance to traffic loads	4.2.7	
[10.2]	Resistance of new bridges to traffic loads: Vertical loads	4.2.7.1.1(1)(a)	6.3.2 (2)P [28]
	Equivalent vertical loading for new geotechnical structures, earthworks and earth pressure effects	4.2.7.2(1)	

[28] If agreed by the competent authority of the Contracting State, it is permitted to design geotechnical structures, earthworks and calculate earth pressure effects using line loads or point loads, where their load effects correspond to the Load Model 71 with factor α.

Index	Characteristics to be assessed	UTP Point	Mandatory standard point
	Capability requirements for existing structures in accordance with traffic code	Appendix E – Load Model 71	
[10.3]	Resistance of new bridges to traffic loads: Vertical loads	4.2.7.1.1(1)(b)	6.3.3 (3)P
	Capability requirements for existing structures in accordance with traffic code	Appendix E – Load model SW/0	
[10.4]	Resistance of new bridges to traffic loads: Vertical loads	4.2.7.1.1(2)	6.3.2 (3)P and 6.3.3 (5)P
	Equivalent vertical loading for new geotechnical structures, earthworks and earth pressure effects	4.2.7.2(2)	
[10.5]	Allowance for dynamic effects of vertical loads	4.2.7.1.2(1)	6.4.3 (1)P and 6.4.5.2 (2)
[10.6]	Allowance for dynamic effects of vertical loads	4.2.7.1.2(2)	6.4.4
[10.7]	Allowance for dynamic effects of vertical loads	4.2.7.1.2(2)	6.4.6.1.1 (3) to (6)
	Capability requirements for existing structures in accordance with traffic code	Appendix E – Load model HSLM	
[10.8]	Centrifugal forces	4.2.7.1.3	6.5.1 (2), (4)P and (7)
[10.9]	Nosing forces	4.2.7.1.4	6.5.2
[10.10]	Actions due to traction and braking (longitudinal loads)	4.2.7.1.5	6.5.3 (2)P, (4), (5), (6).and (7)P
[10.11]	Resistance of new structures over or adjacent to tracks	4.2.7.3	6.6.2 to 6.6.6
[11]	**Annex A2 to EN 1990:2002 issued as EN 1990:2002/A1:2005** **Eurocode – Basis of structural design**		
[11.1]	Structures resistance to traffic loads	4.2.7	
[11.2]	Design track twist due to rail traffic actions	4.2.7.1.6	A2.4.4.2.2(3)P
[12]	**EN 13848-5:2017** **Railway applications – Track – Track geometry quality – Part 5: Geometric quality levels – Plain line, switches and crossings**		
[12.1]	The immediate action limit for alignment	4.2.8.1(1)	7.5 Limits of wavelength range D1 set out in table 5
[12.2]	The immediate action limit for longitudinal level	4.2.8.2(1)	7.3 Limits of wavelength range D1 set out in table 4
[12.3]	The immediate action limit for track twist	4.2.8.3(2)	7.6
[12.4]	The immediate action limit for track twist - 1 668 mm track gauge system	4.2.8.3(6)	Annex C

Index	Characteristics to be assessed	UTP Point	Mandatory standard point
[13]	**EN 13848-1:2019** **Railway applications – Track – Track geometry quality – Part 1: Characterization of track geometry**		
[13.1]	The immediate action limit for track twist	4.2.8.3(1)	6.5
[14]	**EN 14067-5:2021/AC:2023** **Railway applications – Aerodynamics – Part 5: Requirements and test procedures for aerodynamics in tunnels**		
[14.1]	Criterion for new tunnels	4.2.10.1(1)	6.1.3 Table 10
[14.2]	Criterion for existing tunnels	4.2.10.1(3)	6.1.4
[14.3]	Assessment procedure	6.2.4.12(1)	6.1, 7.4
[14.4]	Reference cross section	6.2.4.12(3)	6.1.2.1
[15]	**EN 13145:2001** **Railway applications – Track – Wood sleepers and bearers**		
[15.1]	Resistance to vertical loads	Appendix C.1, point (c) Appendix C.2, point (c)	
[16]	**EN ISO 6506-1:2014** **Metallic materials – Brinell hardness test. Test method.**		
[16.1]	Definition of steel hardness	Appendix S	
[17]	**EN 13232-3:2003** **Railway applications – Track – Switches and crossings – Part 3: Requirements for wheel/rail interaction**		
[17.1]	Definition of the “unguided length of an obtuse crossing”	Appendix S	4.2.5

Table 50 Technical Documents (available on ERA website)

Index	Characteristics to be assessed	UTP Point	Mandatory standard point
[A]	**ERA Technical Document on codification of combined transport ERA/TD/2023-01/CCT version 1.1 (released on 2023-03-21)**		
[A.1]	Codification of lines	2.6	2.1